

Laurance Lake Temperature Model



by

Christopher J. Berger,

Scott A. Wells

And

Robert Annear

Maseeh College of Engineering and Computer Science
Department of Civil and Environmental Engineering
Portland State University
Portland, Oregon 97201-0751

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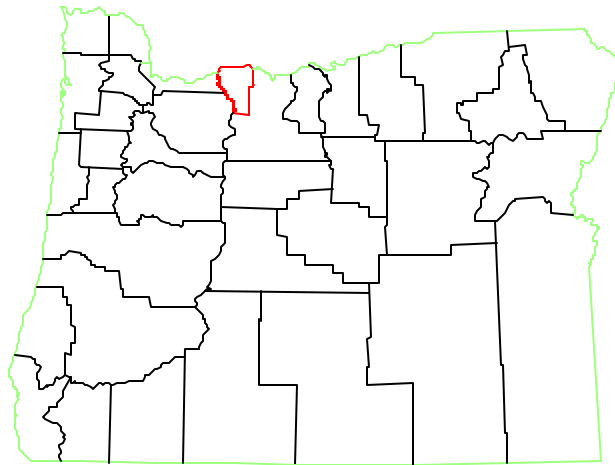
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Introduction

Laurance Lake is a reservoir located in Hood River County, Oregon (Figure 1). It is located at the base on Mt. Hood in Oregon (see Figure 2 and Figure 3), discharges into the Middle Fork of the Hood River. The reservoir was constructed in 1968 for irrigation storage and has a capacity 3564 acre-feet at full pool. Since the river violates temperature standards, this study has been designed to construct a hydrodynamic and temperature model of Laurance reservoir in order to assess strategies for improving temperatures in the Middle Fork River.



*POPULATION OF HOOD RIVER COUNTY
Approximately 20,000*

Figure 1. Hood River County, Oregon.

The objectives of the study are then to

- ❑ Develop a hydrodynamic and temperature model of Laurance Lake
- ❑ Calibrate the model to field data collected from November 2002 through Spring 2004
- ❑ Use the model to evaluate strategies for temperature improvement through operational or structural changes to Lake Laurance

The model chosen for development was CE-QUAL-W2 Version 3.2 (Cole and Wells, 2004). This is a two-dimensional unsteady hydrodynamic, temperature and water quality model that includes typical eutrophication parameters (algae, nutrients, temperature, organic matter, dissolved oxygen, pH). PSU, under the support of the Corps of Engineers Waterways Experiment Station, is a center for development of this modeling tool.

In order to model the system, the following data were required:

- Laurance Lake outflow, water level and temperature data at the upstream system boundary (Clear Creek)

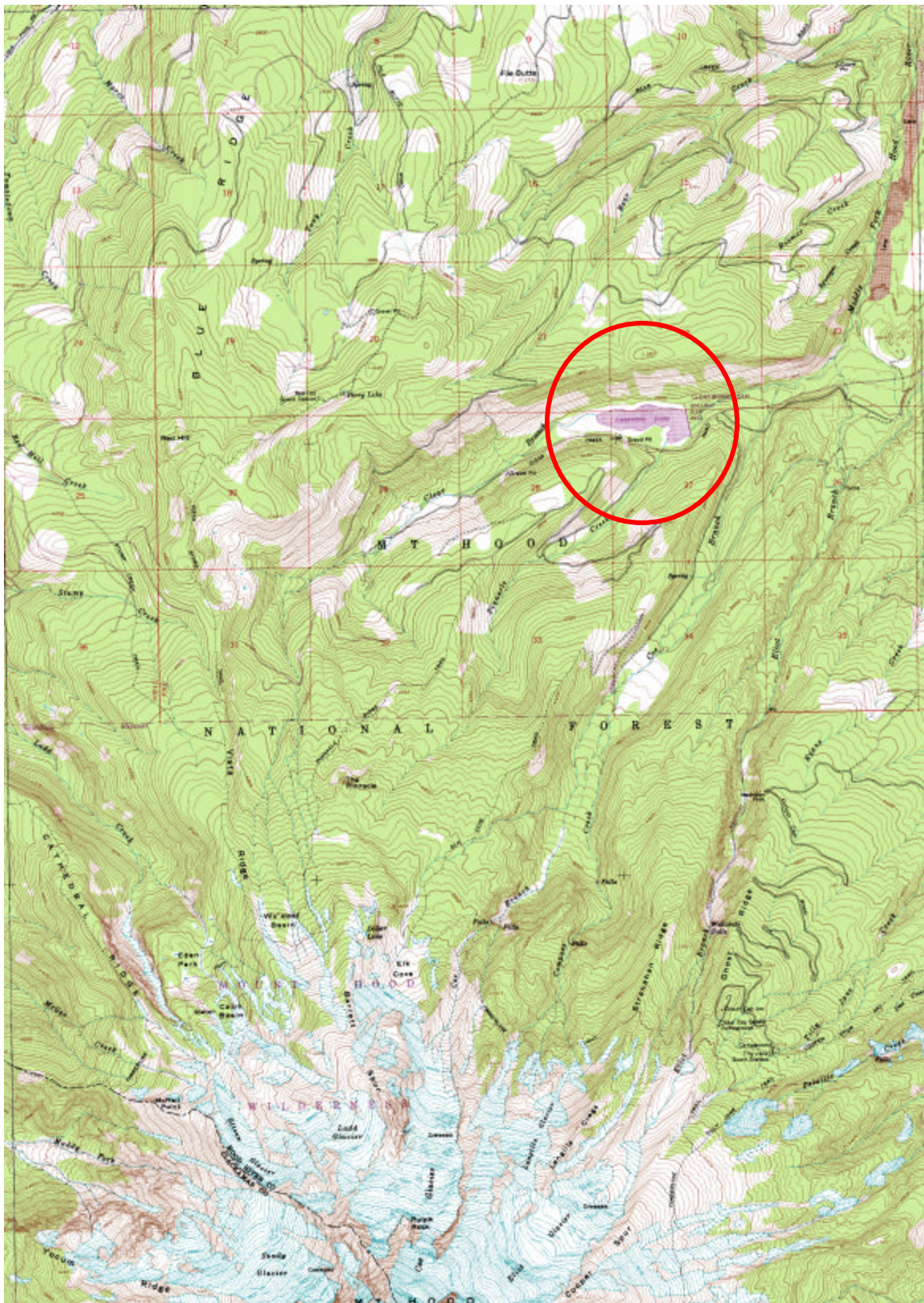


Figure 2. Lake Laurance, Oregon.

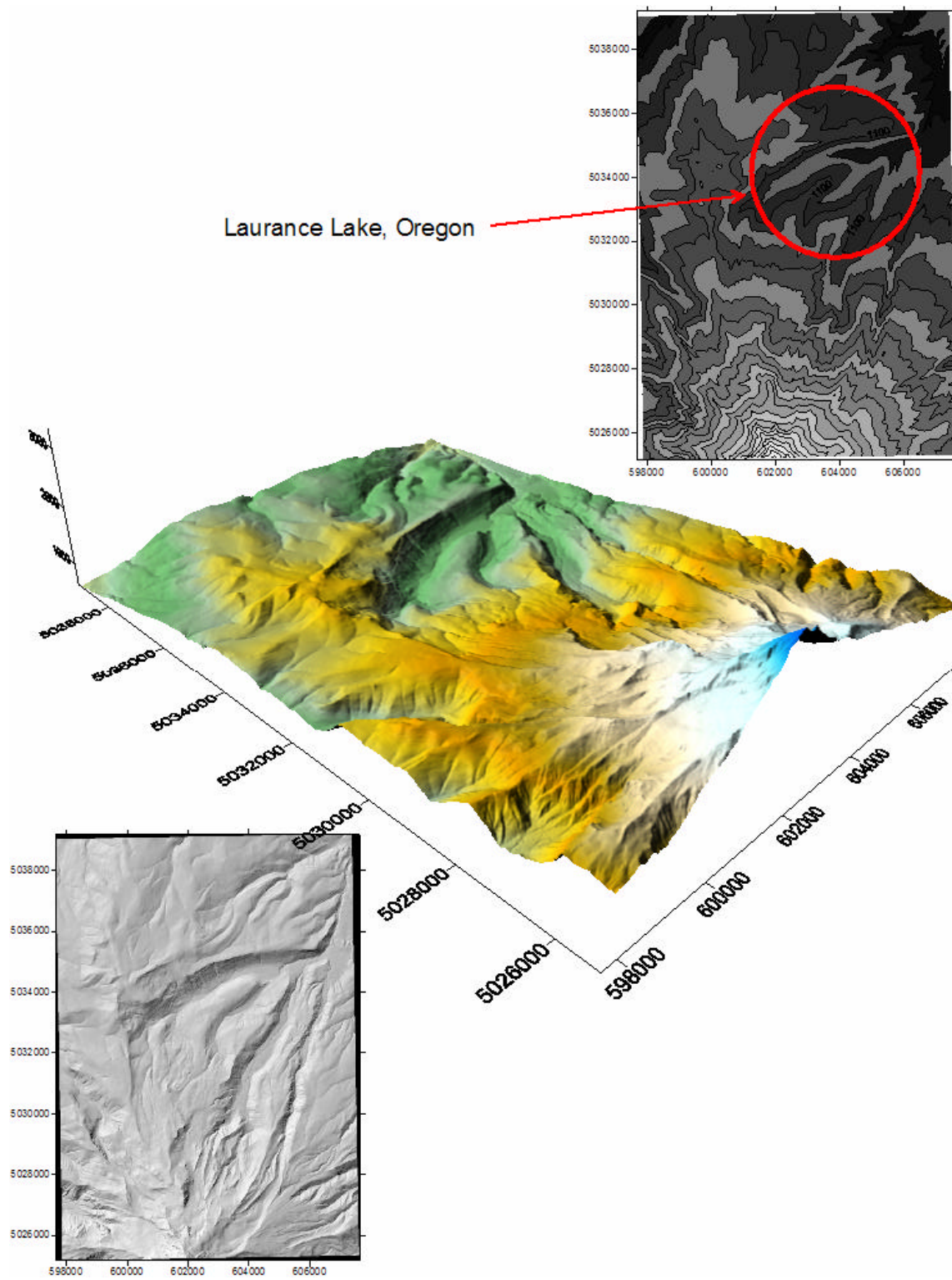


Figure 3. Topography around Laurance Lake.

- Tributary inflows and temperatures
- Meteorological conditions
- Bathymetry of Laurance Lake

Data have been primarily collected from 2002 to 2004. This report summarizes model development. Information provided in this report was organized in the following sections:

- Model Selection
- Model Forcing Data
- Hydrodynamic Calibration
- Temperature Calibration
- Management Scenarios
- Summary and Conclusions

Also discussed are issues relative to the calibration effort. Calibration focused on model predictions of hydrodynamics (flow and water level) and temperature. The model calibration period was from May 1, 2003 to April 30, 2004.

Background Information

The following information from Oregon DEQ and Middle Fork Irrigation District documents temperature issues in the Laurance Lake system (ODEQ and MFID, 2002):

“The waters in Clear Branch and the Middle Fork Hood River below Clear Branch Dam have been identified as water quality limited for temperature and placed on the 303(d) list as required by the Federal Clean Water Act. Clear Branch and the Middle Fork Hood River are included on the 303 (d) list for exceeding the State of Oregon’s Bull trout (*Salvelinus confluentus*) criterion of 10° C. Bull trout inhabit Clear Branch and the Middle Fork Hood River and were listed as a threatened species under the Endangered Species Act in 1998. Clear Branch Dam was constructed under P.L. 566 with the help of the NRCS and is operated and maintained by Middle Fork Irrigation District (MFID). Clear Branch and Coe Branch join together about 0.5 miles below the Clear Branch Dam to form the Middle Fork Hood River.”

“The Western Hood Subbasin Total Maximum Daily Load (TMDL) was approved by EPA in January 2002 and lists the “critical period” for Clear Branch below Laurance Lake as year round. Suggested solutions to this temperature problem have included diverting colder Pinnacle Creek water to the base of the dam or a selective withdrawal system in the Lake. MFID is required to develop and implement a “Surface Water Temperature Management Plan” for the operation of the Clear Branch Dam by Oregon Department of Environmental Quality (DEQ). Temperature data has been collected above and below Laurance Lake since 1997. That data is not complete. Flow data for Pinnacle and Clear Branch Creeks feeding Laurance Lake is not available. Anecdotal evidence indicates there is a groundwater influence in Laurance Lake. There are springs at the base of the dam, one has been monitored for temperature and frequently exceeds the 10° C standard. Before a “Surface Water Temperature Management Plan” can be created and a computer model run to examine temperature and flow dynamics, cooling/warming effects of Pinnacle and Clear Branch creeks and ground water influence, more data must be acquired.”

Model Selection

Selection of the appropriate water quality model is a function of properly identifying the water quality problem ("conceptualization") and selecting a model which appropriately describes the water quality changes in the water body, is theoretically valid, and can be easily adapted to site-specific physical characteristics of the water body.

The performance of a mathematical model in predicting the existing and future water quality dynamics of a system is dependent on the following steps:

- (i) identification of the problem
- (ii) selection of model type and relationship of model to the problem
- (iii) computational representation
- (iv) model response studies or model sensitivity analyses
- (v) model calibration
- (vi) application of model to evaluate management strategies

Because there are many water quality models available, a choice of the appropriate model would be made after considering the following questions: What physical processes are represented in the model and which are ignored? How are physical processes included in the model? What processes are represented by model coefficients? For example in defining the problem, the following questions could be asked:

- (i) What are the dominant physical processes at work and can the chosen model represent those processes? (such as, how does the water move? Is there stratification, wind-driven currents, and/or selective withdrawal?)
- (ii) What are the spatial and temporal scales of these processes and can the model represent them? (such as, is steady-state representation adequate, is 1-D, 2-D, or 3-D spatial discretization necessary?)

The choice of the proper model is also based on answering

- (1) site specific questions (physical characteristics of the each system component - river or reservoir reach, water quality cycles, algal types),
- (2) management objectives (required accuracy, use for future studies),
- (3) project resources (data availability, staff constraints, time limitations).

The model chosen for Laurance Lake was the Corps of Engineers model CE-QUAL-W2 Version 3.2. CE-QUAL-W2 Version 3.2 is a dynamic 2-d (x-z) model developed for stratified water-bodies (Cole and Wells, 2004). This is a Corps of Engineers modification of the Laterally Averaged Reservoir Model (Edinger and Buchak 1978). CE-QUAL-W2, whose grid is shown in Figure 4, consists of directly coupled hydrodynamic and water quality transport models. Hydrodynamic computations are influenced by variable water density caused by temperature, salinity, and dissolved and suspended solids. Developed for reservoirs and narrow, stratified estuaries, CE-QUAL-W2 can handle a branched and/or looped system with flow and/or head boundary conditions. With two dimensions depicted, point and non-point loading can be spatially distributed. Relative to other 2-D models, CE-QUAL-W2 is efficient

and cost effective to use. This model allows the user to use the ultimate quickest Numerical Scheme for improved numerical accuracy.

In addition to temperature, CE-QUAL-W2 Version 3.2 can simulate many water quality variables. Primary physical processes included are surface heat transfer, short-wave and long-wave radiation and penetration, convective mixing, wind and flow induced mixing, entrainment of ambient water by pumped-storage inflows, inflow density stratification as impacted by temperature and dissolved and suspended solids. Major chemical and biological processes in CE-QUAL-W2 include: the effects of DO of atmospheric exchange, photosynthesis, respiration, organic matter decomposition, nitrification, and chemical oxidation of reduced substances; uptake, excretion, and regeneration of phosphorus and nitrogen and nitrification-denitrification under aerobic and anaerobic conditions; carbon cycling and alkalinity-pH-CO₂ interactions; trophic relationships for total phytoplankton; accumulation and decomposition of detritus and organic sediment; and coliform bacteria mortality.

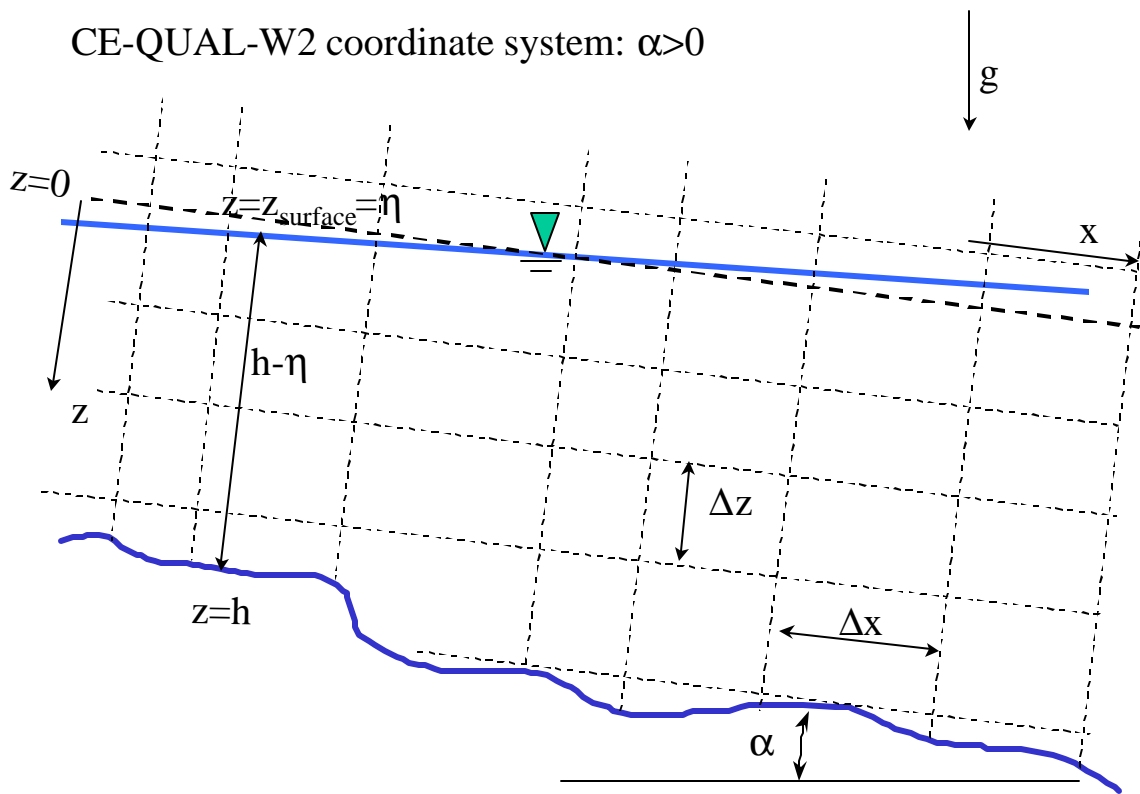


Figure 4. Coordinate system for CE-QUAL-W2 Version 3.2.

Models, such as WQRSS (Smith 1978), HEC-5Q (Corps of Engineers 1986), and HSPF (Donigian, et al. 1984), have been developed for river basin modeling but have serious limitations. One issue is that the HEC-5Q (similar to WQRSS) and HSPF models incorporate a one-dimensional, longitudinal river model with a one-dimensional, vertical reservoir model (one-dimensional for temperature and water quality and zero dimensional for hydrodynamics). The modeler must choose the location of the transition from 1-D longitudinal to 1-D vertical. Besides the limitation of not solving for the velocity field in the stratified, reservoir system, any point source inputs to the reservoir section are spread over the entire longitudinal distribution of the reservoir layer.

Also, other one-dimensional reservoir models, such as the HEC WQRRS (Water Quality River-Reservoir Simulation) model and the Corps's CE-QUAL-R1, are also not adequate to compute 2-D circulation within pool areas. These models conceptualize a pool as well mixed in each horizontal slab, i.e., over the length and the width of the system. By making this assumption, the vertical and longitudinal circulation patterns within a pool cannot be resolved.

Based on the depth Laurance Lake, a one-dimensional reservoir model of the river system would not be adequate because of possible longitudinal and vertical gradients in water quality.

For this project, the CE-QUAL-W2 River Basin Model Version 3.2 (as schematized in Figure 5) was the most appropriate for modeling Laurance Lake since it contains the following elements:

- Two-dimensional, dynamic hydrodynamics and water quality capable of replicating any density stratified environment.
- The hydraulic elements at the dam (outlet pipe and spillway) can be accurately represented
- The model is a state-of-the-art tool with features not found in other models

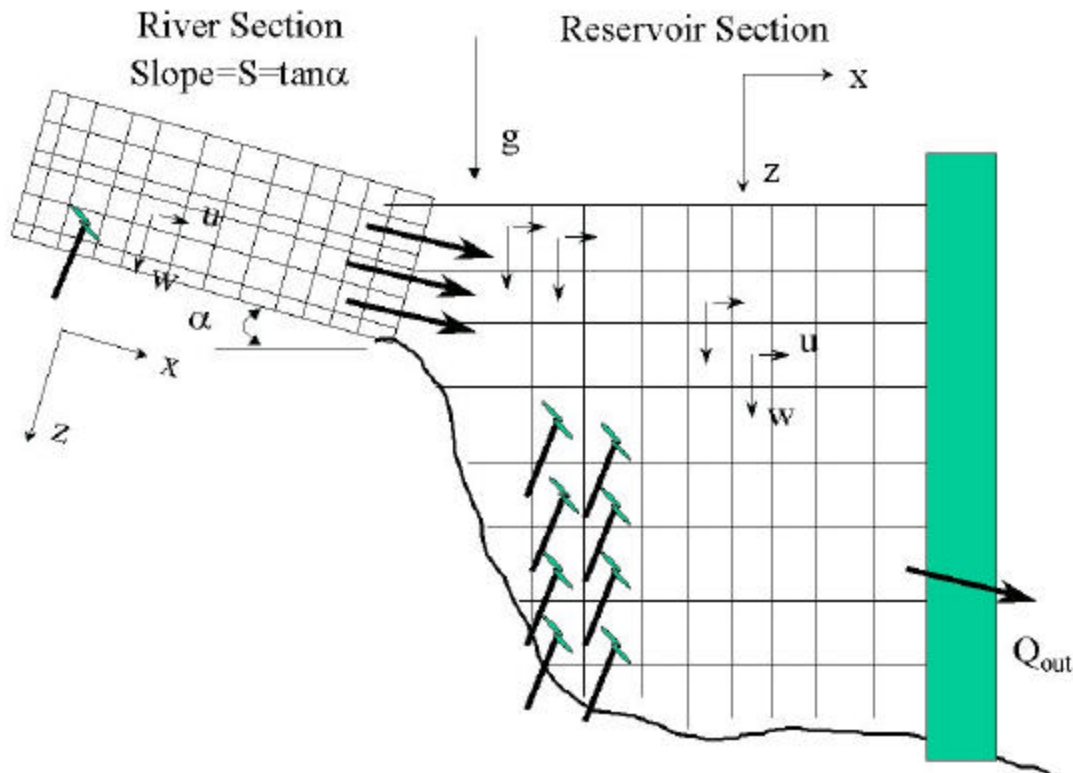


Figure 5. Conceptual schematic of river-reservoir connection in CE-QUAL-W2 Version 3.

This model has been under development for many years and is a public-domain code maintained by the Corps of Engineers, Waterways Experiments Station (WES), located in Vicksburg, Mississippi. Version 3.2 has and is undergoing rigorous testing and has been successfully applied to many river basin systems. Further information about CE-QUAL-W2 Version 3 is shown at <http://www.ce.pdx.edu/w2>.

Model Forcing Data

The model forcing data consists of the system bathymetry developed into the model grid; the boundary condition flow and temperature; the tributary and flow and temperature; and the system meteorology.

Water quality monitoring sites from which data were used for model development were identified in Figure 6 and were described in Table 1.

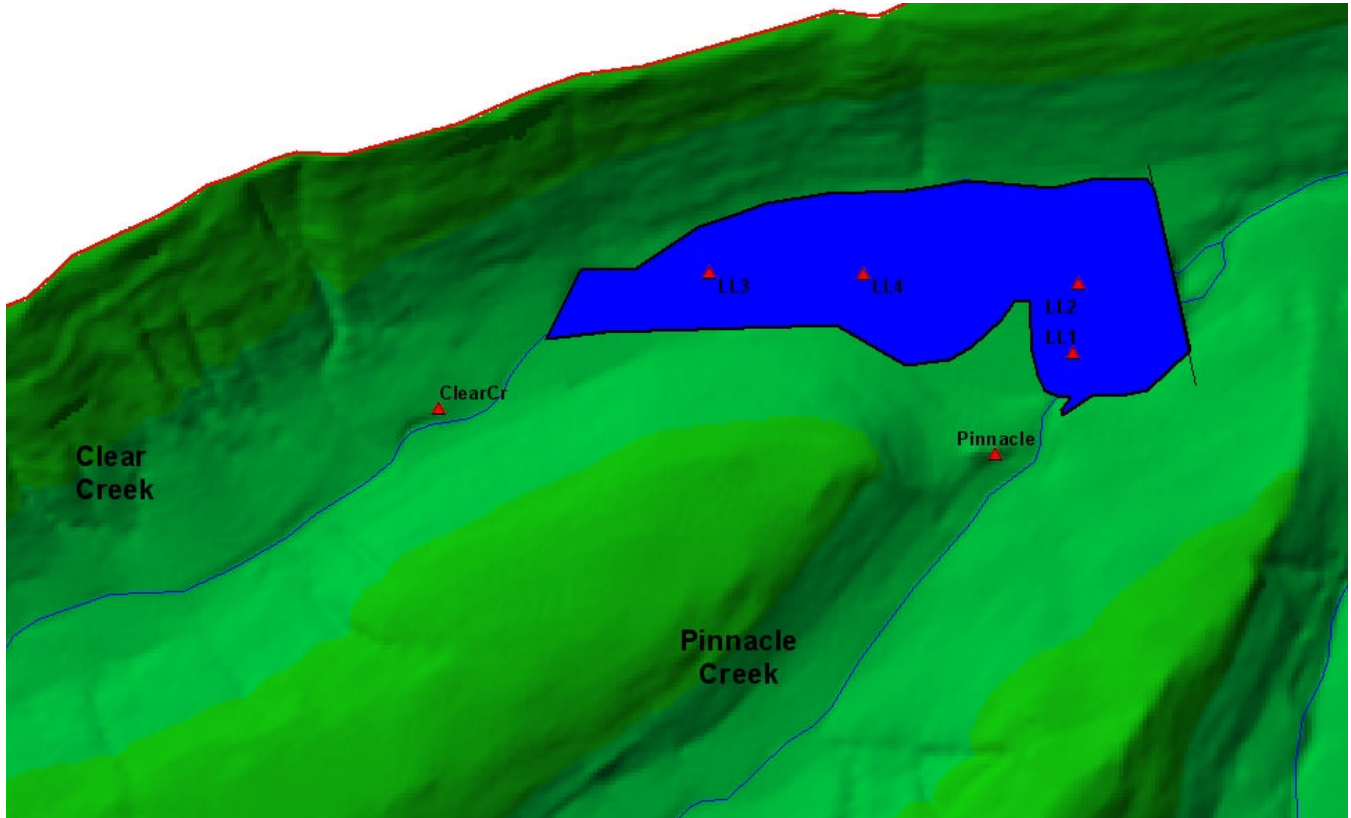


Figure 6. Monitoring sites at Laurance Lake

Table 1. Monitoring sites	
Site ID	Description
CC	Clear Creek above Reservoir
PC	Pinnacle Creek above Reservoir
LL1	Laurance Lake at Pinnacle Creek Branch
LL2	Laurance Lake near dam
LL3	Laurance Lake, middle
LL4	Laurance Lake near upstream end

Model Geometry

Laurance Lake Bathymetry

The Long Lake bathymetry was developed using depth soundings, a USGS digital elevation map (DEM), and a bathymetric contour map provided by Middle Fork Irrigation district. The data points used to develop the bathymetry were shown in Figure 7. Model bathymetry was created up to an elevation of 925 meters, 17 meters above the current full pool elevation, to allow the simulation of management scenarios that included raising the dam. In general, data from depth soundings were used to describe bathymetry below current full pool elevations, the bathymetric contour map was used for areas near the bank, and the USGS DEM data were used for elevations well above the full pool elevation.

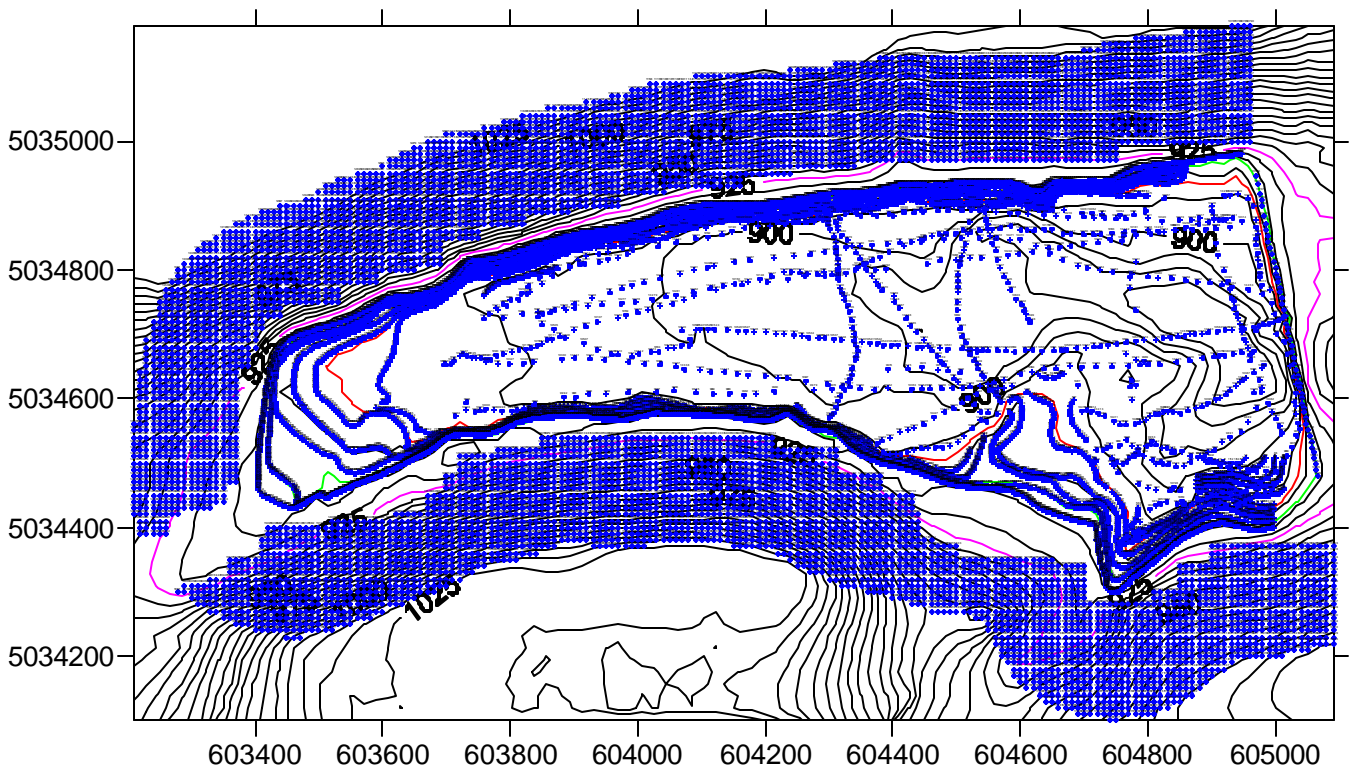


Figure 7. Location of data points used to develop bathymetry

Grid Layout

Figure 8 shows the plan view of the grid layout for the Laurance Lake.

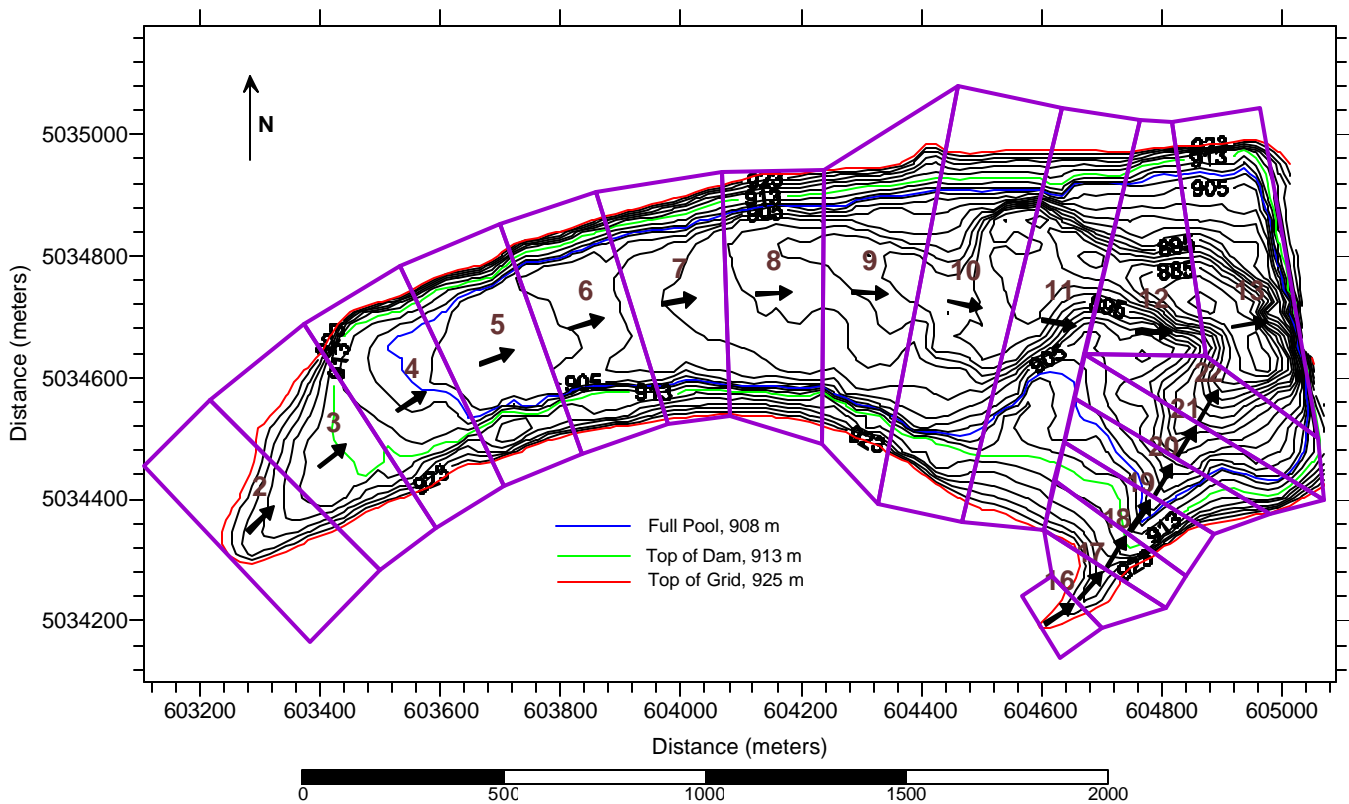


Figure 8. Plan view of the Laurance Lake grid. The arrows show the segment orientation.

The model was divided into two branches, the first representing Clear Creek and the second Pinnacle Creek. Branch 1 had 12 active segments and branch 2 contained 7 active segments. The model segments in branch 1 had a length of 159.6 meters whereas those in branch 2 were 71.3 meters long. There were a total of 90 active model layers each having a thickness of 0.5 meters. Model layers for branch 1 were shown in Figure 9.

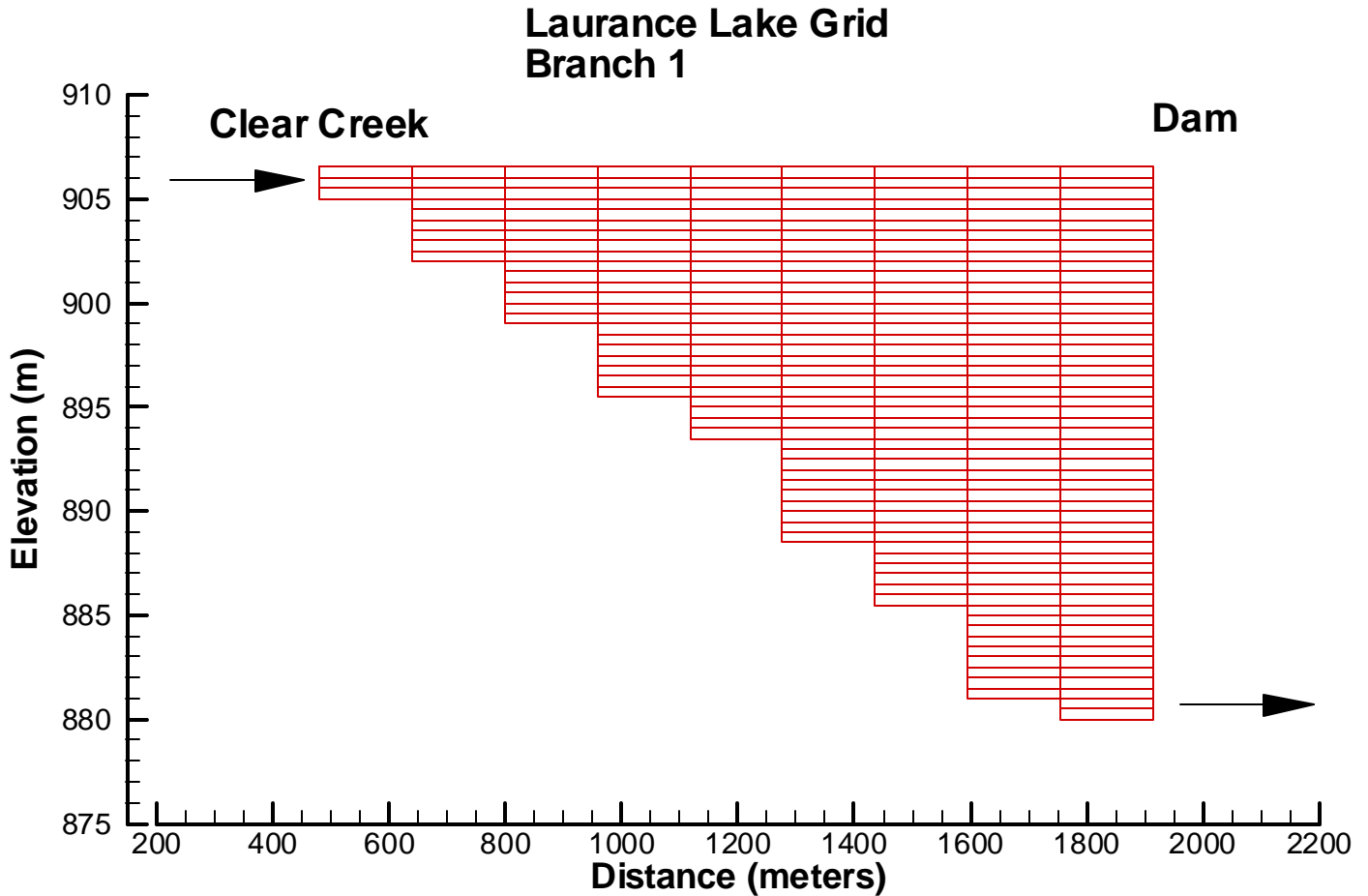


Figure 9. Layer elevations of branch 1. Only layers below the full pool elevation were shown.

In the CE-QUAL-W2 model, the model user must specify the characteristics and connectivity of the model grid. The following parameters were used in the Laurance Lake model (see Cole and Wells, 2004, for detailed explanation of model grid characteristics):

- IMP (# of segments): 23
- KMP (# of vertical layers): 92
- NWB (# of water-bodies): 1
- NBR (# of branches): 2

The branch layout was specified by these parameters for each branch (as specified in the w2_con.npt control file – see Cole and Wells, 2004).

Boundary Conditions

The upstream boundary condition for the Laurance Lake model was Clear Creek. Clear Creek inflows were based on gaging station data and a regression equation developed from a correlation between Clear Creek and Pinnacle Creek flow rates (Figure 10). The regression equation was only used when Clear Creek gaging data were not available. Figure 11 shows the flow rates used for Clear Creek.

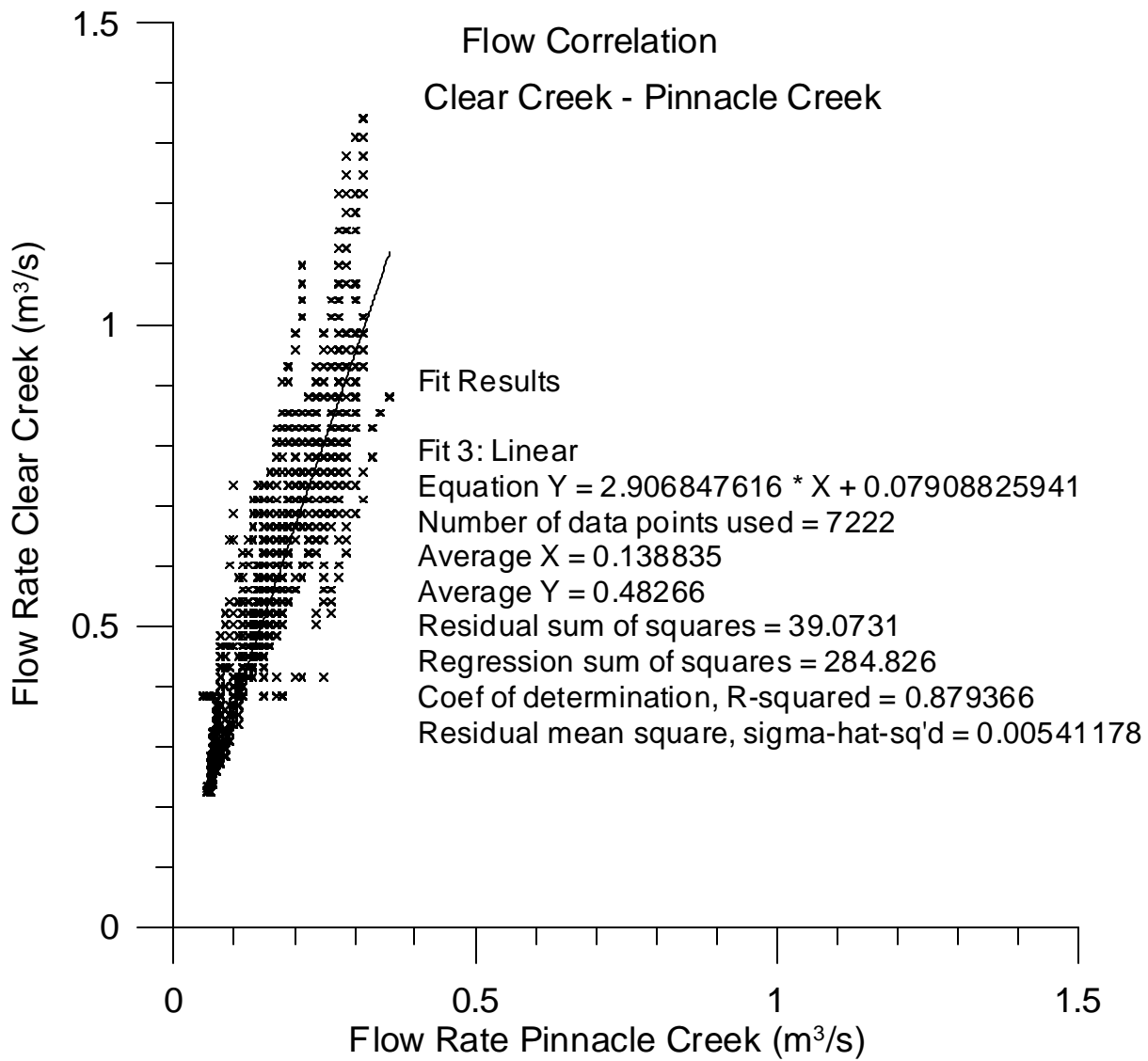


Figure 10. Correlation between Clear Creek and Pinnacle Creek.

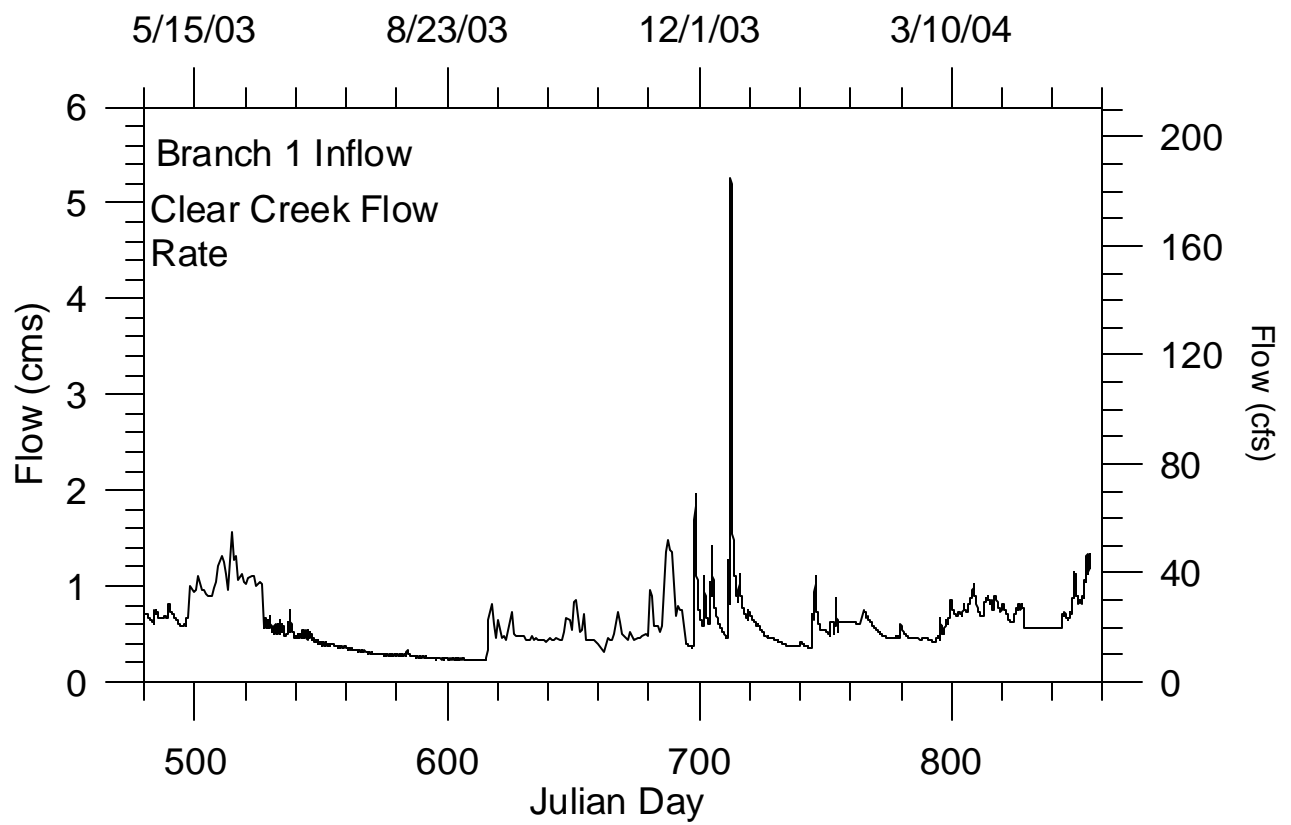


Figure 11. Clear Creek flow rates.

Clear Creek inflow temperatures were based on measured data obtained from a sampling site near the reservoir. Figure 12 shows a plot of inflow temperatures versus time.

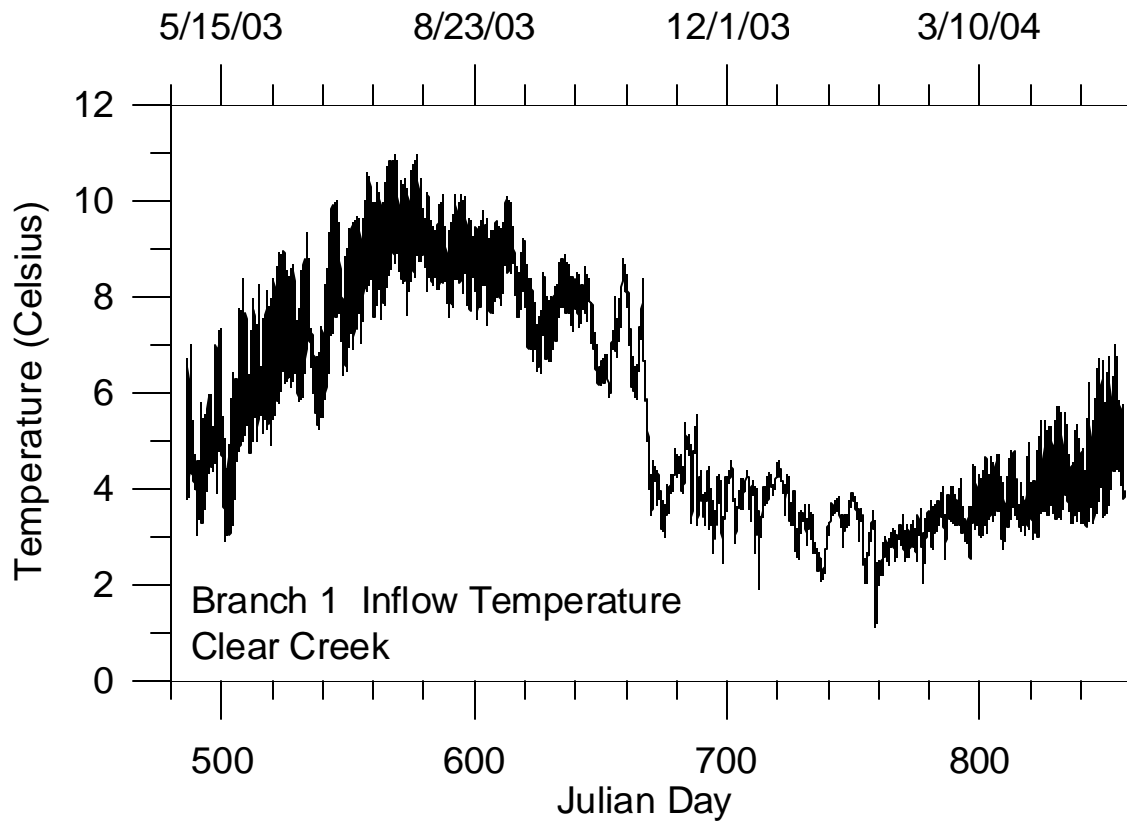


Figure 12. Clear Creek inflow temperatures.

Laurance Lake outflow

The downstream boundary condition was the outflow from Laurance Lake. An outflow record was developed for the model as shown in Figure 13. The outflow file was developed from data provided by Middle Fork Irrigation District (MFID). The outlet pipe is at the bottom of the reservoir adjacent to the dam and was modeled as a point sink.

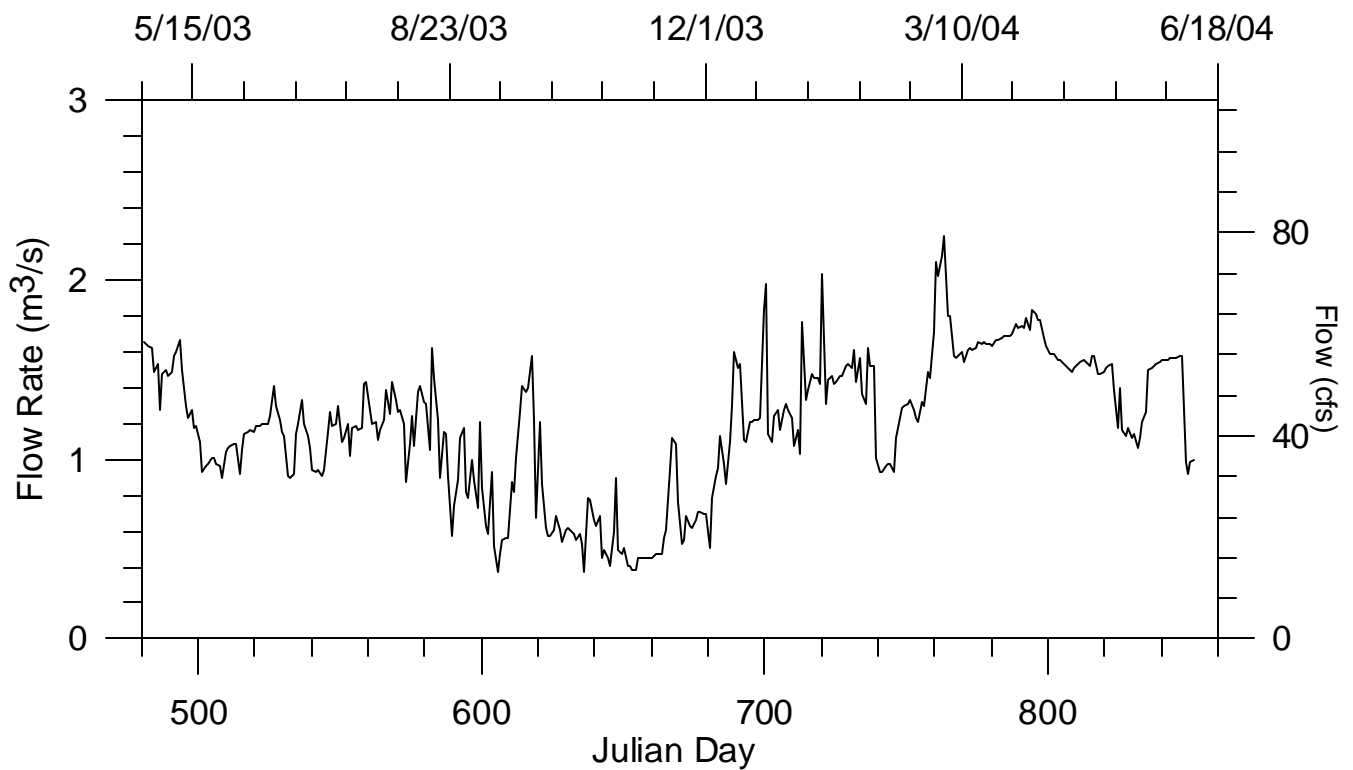


Figure 13. Laurance Lake Outflow

Tributaries

Pinnacle Creek

Pinnacle Creek inflows were shown in Figure 14. Flow rates were developed from gaging station data and a regression equation for time periods when gaging station data did not exist. A regression between Pinnacle Creek and Clear Creek was used during these periods. The correlation between Pinnacle Creek and Clear Creek was shown in Figure 15. Water temperatures used to represent Pinnacle Creek were measured data and were plotted in Figure 16.

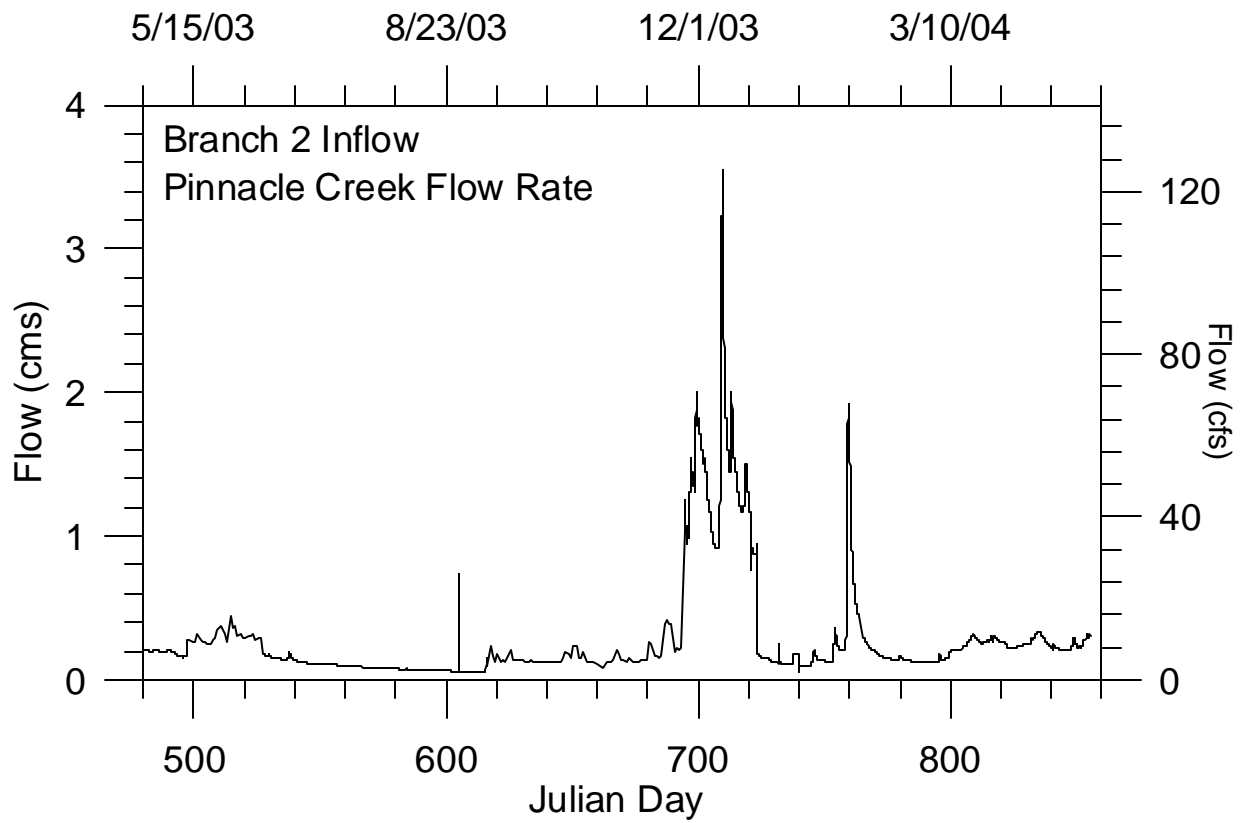


Figure 14. Pinnacle Creek Flow

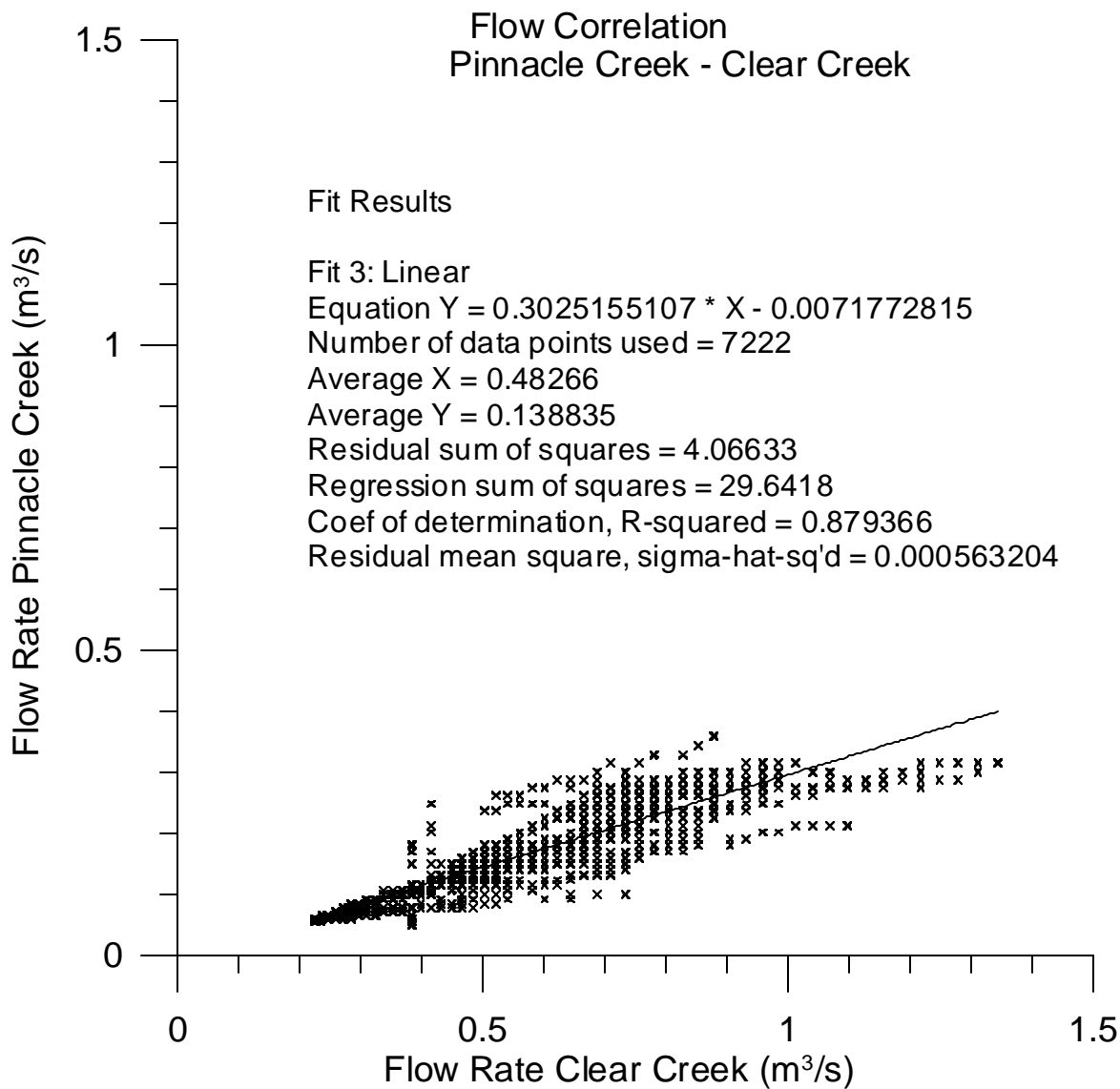


Figure 15. Correlation between Pinnacle Creek and Clear Creek.

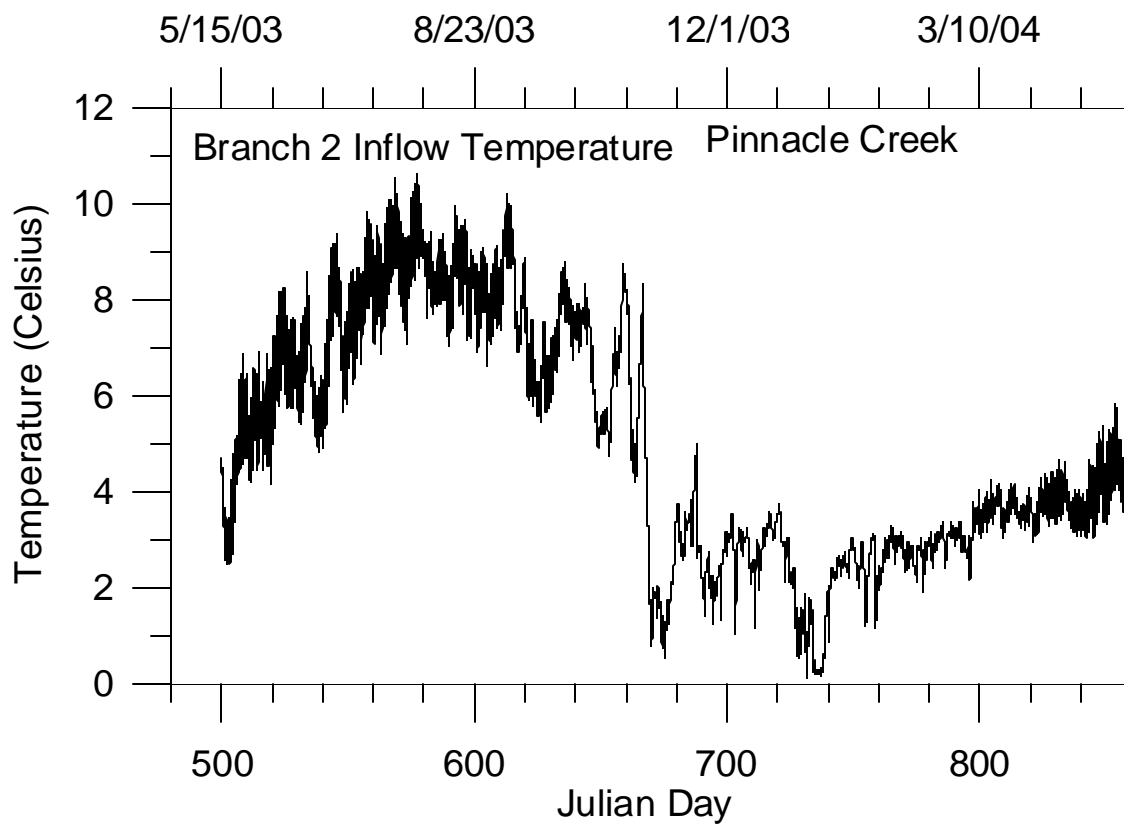


Figure 16. Pinnacle Creek water temperatures

Meteorological Data

Meteorological data for the CE-QUAL-W2 model were measured at the dam and at an agrimet station located at Parkdale. The model utilizes air and dew point temperature, wind speed and direction, and cloud cover or solar radiation. Wind data measured at the dam were accurate only until 8/3/03 (Julian Day 583). Afterwards Parkdale wind speed data were used and wind direction was set to an angle parallel to the axis of the reservoir. Parkdale wind direction data were not used because a comparison between Laurance Lake and Parkdale wind direction showed significant differences. A rose diagram of Parkdale wind direction frequency was shown in Figure 17 and a diagram of Laurance Lake wind direction was shown in Figure 18. Wind at the dam was directed primarily along the axis of the reservoir (see Figure 19 showing the wind rose at the dam superimposed on the lake) whereas the predominant wind directions at Parkdale were from the south and the southwest. Since wind direction data measured at Parkdale were not applicable to Laurance Lake, wind directions for the period when data at the dam did not exist were set to a value of 4.55 radians, which was parallel to the axis of the reservoir. The meteorological station at the dam failed completely on 9/9/03, and afterwards only air temperature, dew point temperature, cloud cover, and short wave radiation data from Parkdale were used. After 11/6/03 (Julian Day 675) air and dew point temperatures regression equations correlating data measured at Laurance Lake and Parkdale were applied. Figure 20 shows the scatter plot and regression equation for air temperature. Figure 21 shows the plot for dew point temperature. The regression equation was not applied before 11/6/03 because using unadjusted Parkdale air and dew point temperature improved model calibration. The regressions were based on data measured over the entire year may not capture the effect of the seasonal trend in air temperature differences during late summer and early fall.

Air and dew point temperatures were shown in Figure 22 and Figure 23, respectively. Figure 24 shows wind speed and Figure 25 shows wind direction relative to time. Cloud cover was plotted in Figure 26.

Wind Direction 2003
Parkdale Agrimet Station
Radians
0.00

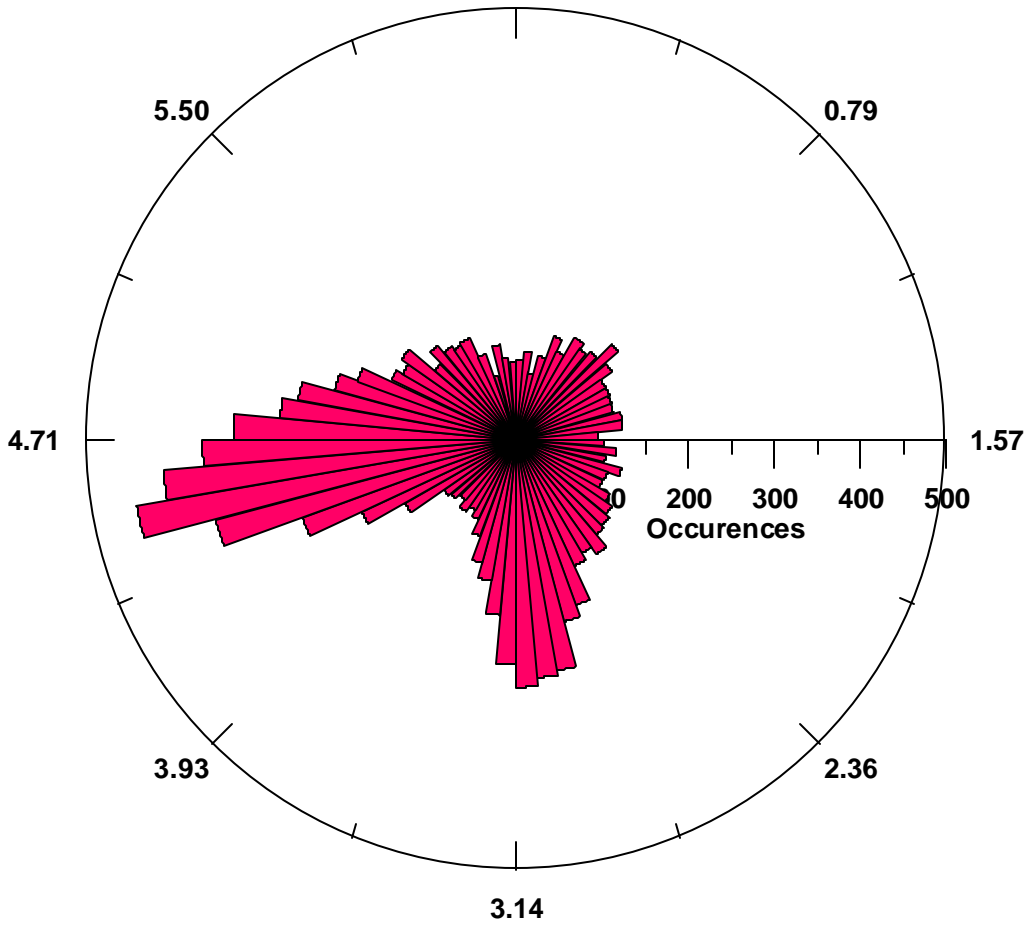


Figure 17. Parkdale wind direction

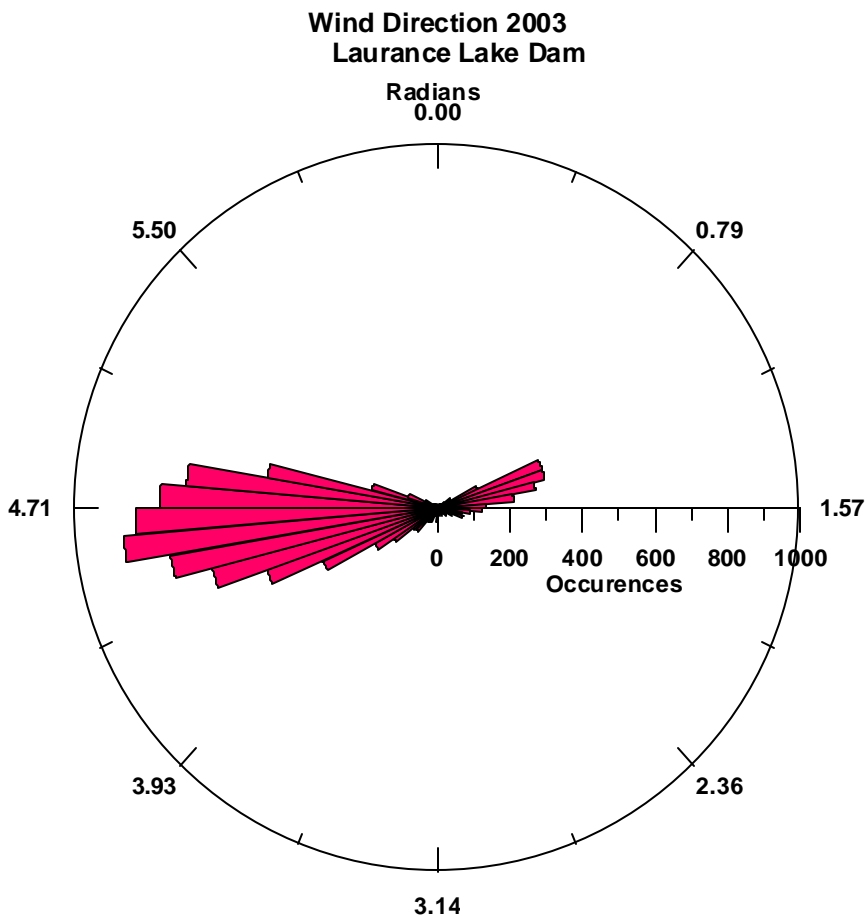


Figure 18. Laurance Lake wind direction

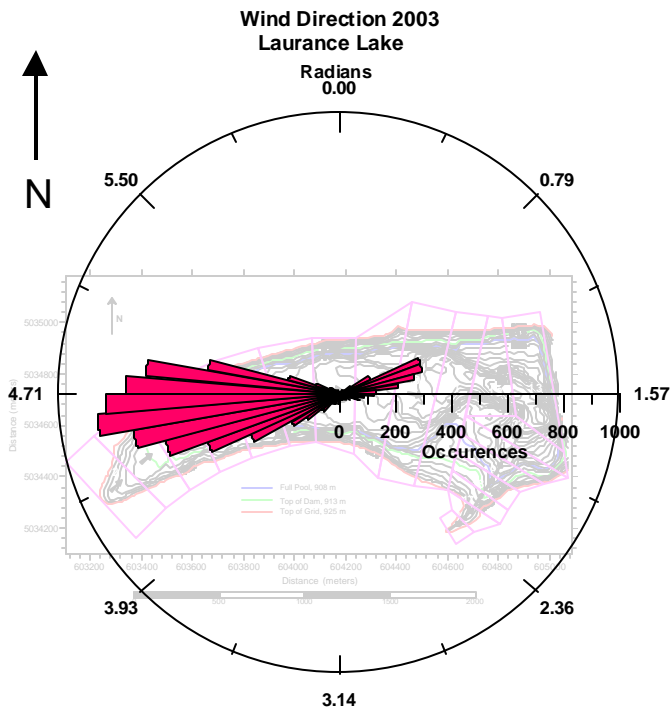
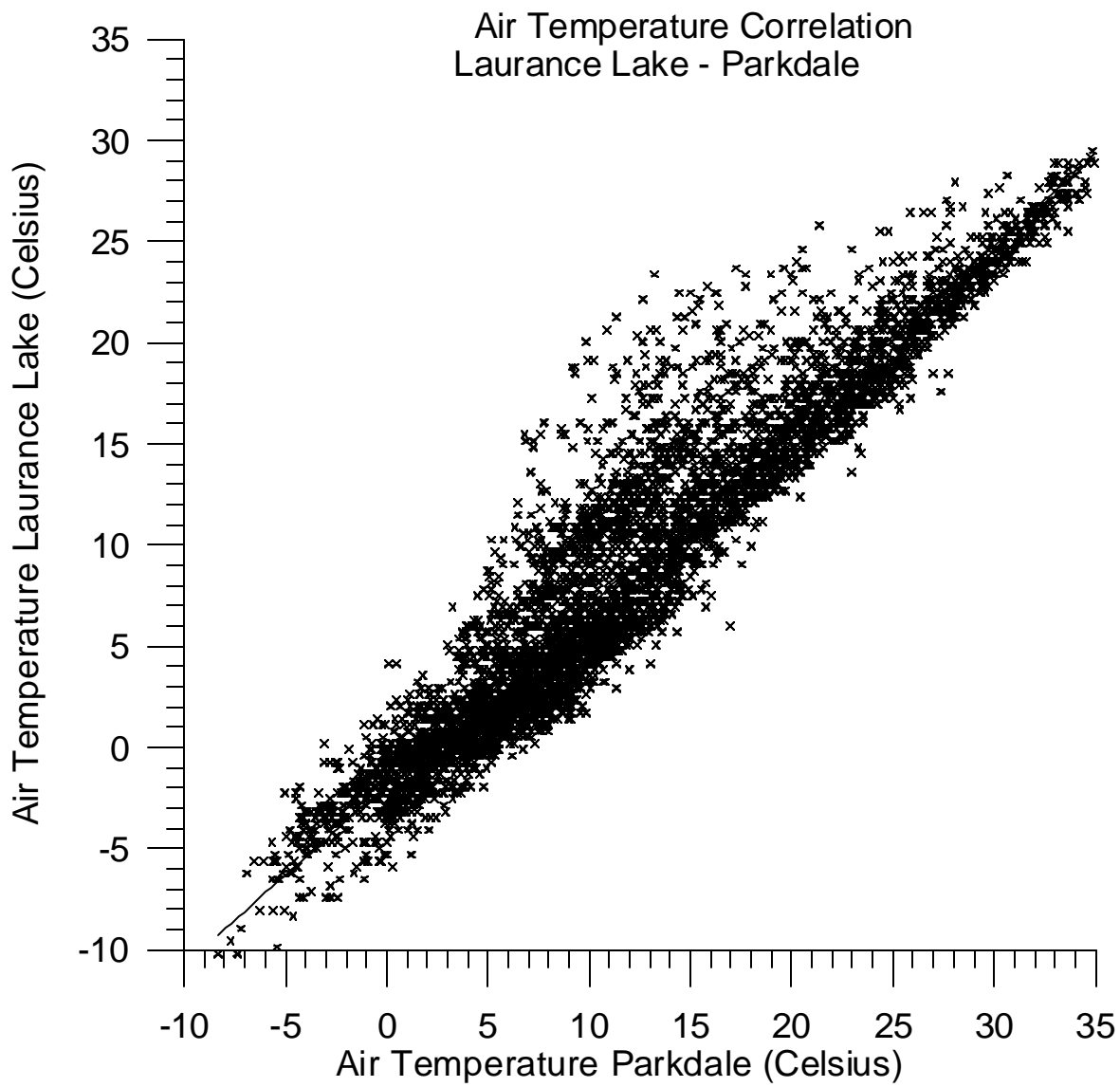


Figure 19. Laurance Lake wind direction at the dam superimposed over the lake axis.



Fit Results

Fit 3: Linear

Equation $Y = 0.888659462 * X - 1.86132248$

Number of data points used = 5682

Average X = 11.6047

Average Y = 8.45128

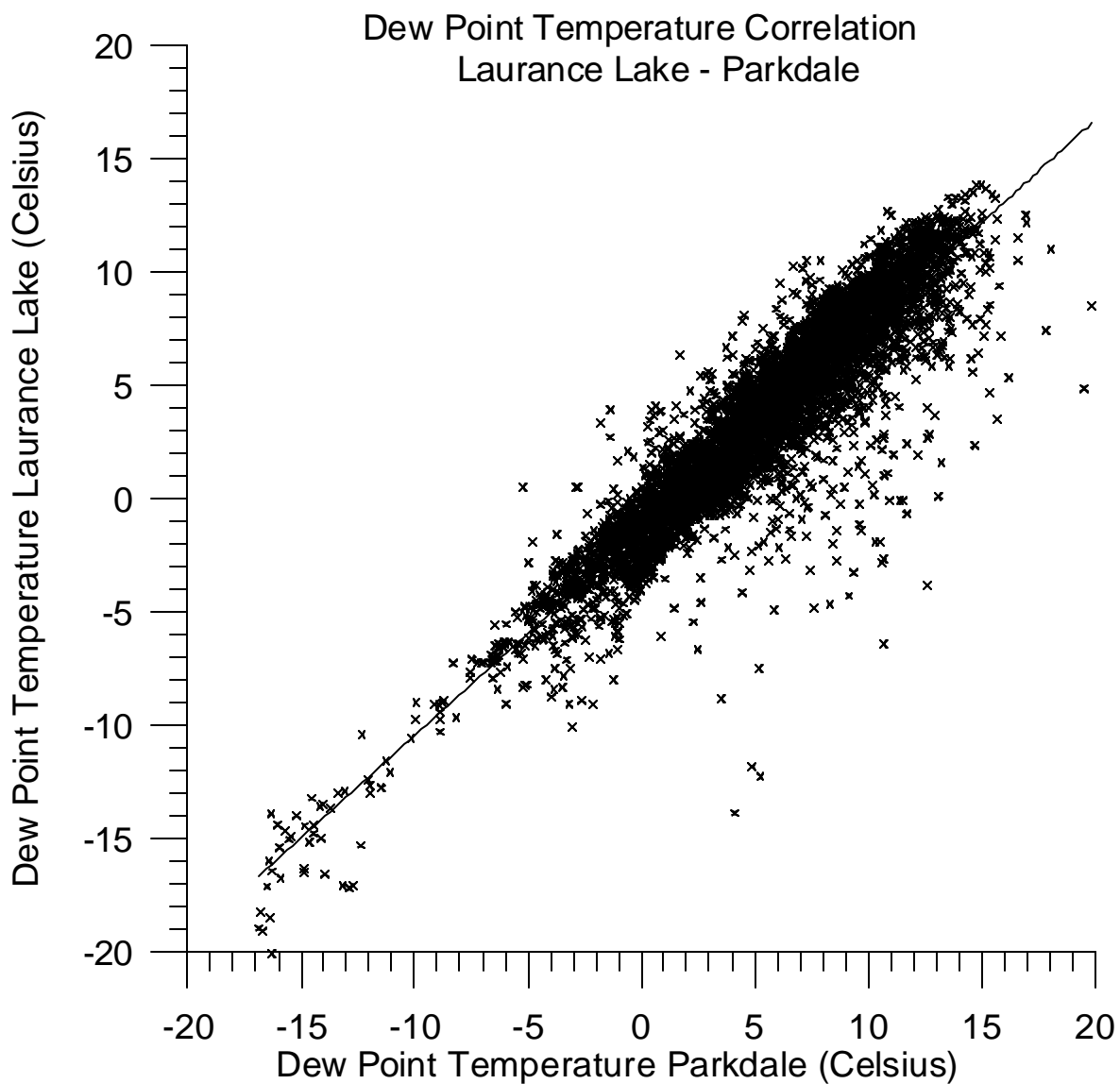
Residual sum of squares = 36308.4

Regression sum of squares = 332031

Coef of determination, R-squared = 0.901427

Residual mean square, sigma-hat-sq'd = 6.39232

Figure 20. Scatter plot of Laurance Lake and Parkdale air temperatures.



Fit Results

Fit 3: Linear

Equation $Y = 0.905115315 * X - 1.396176783$

Number of data points used = 5697

Average X = 5.18157

Average Y = 3.29374

Residual sum of squares = 18243.1

Regression sum of squares = 107586

Coef of determination, R-squared = 0.855017

Residual mean square, sigma-hat-sq'd = 3.20336

Figure 21. Scatter plot of Laurance Lake and Parkdale dew point temperatures.

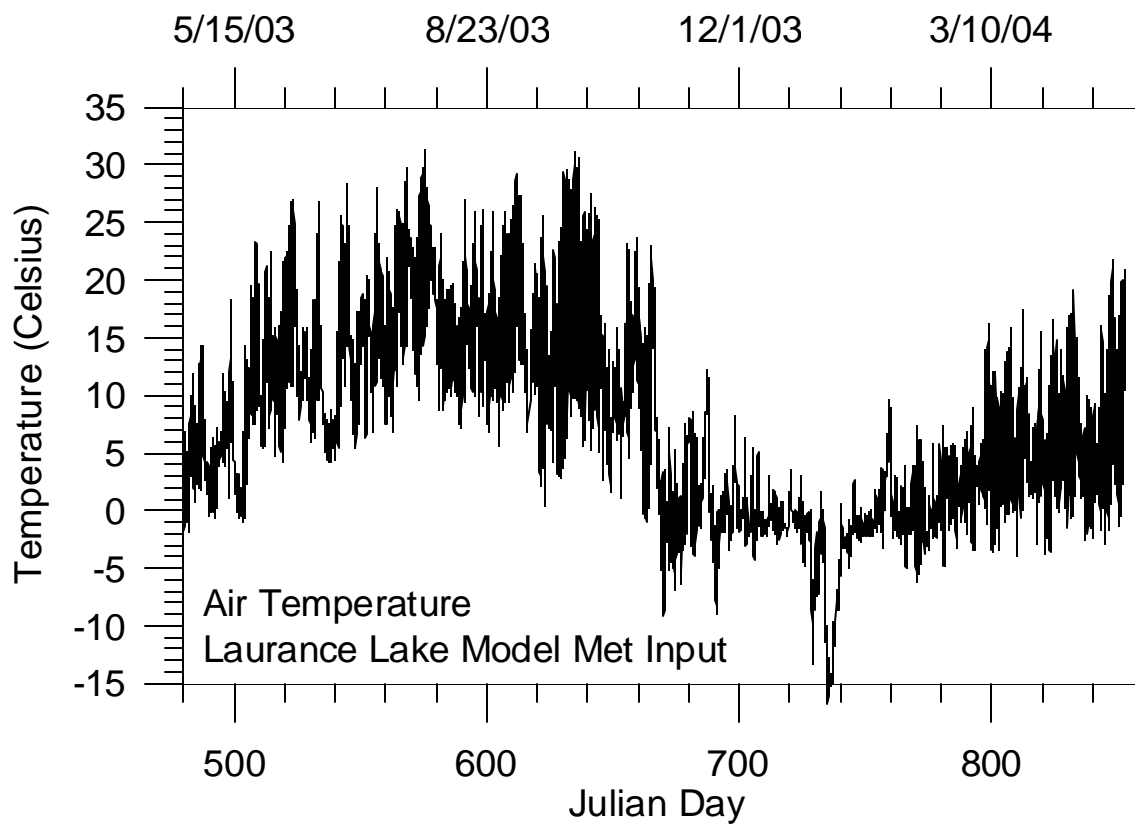


Figure 22. Air temperature, °C

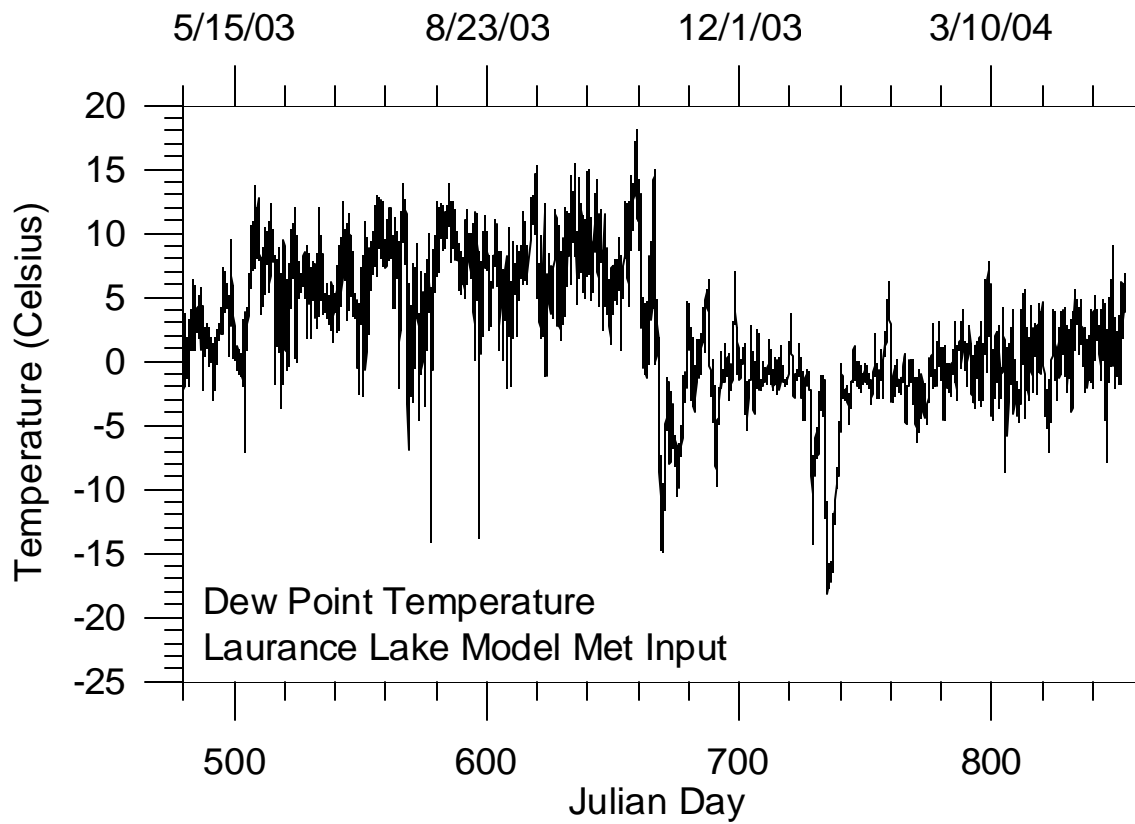


Figure 23. Dew point temperature, °C

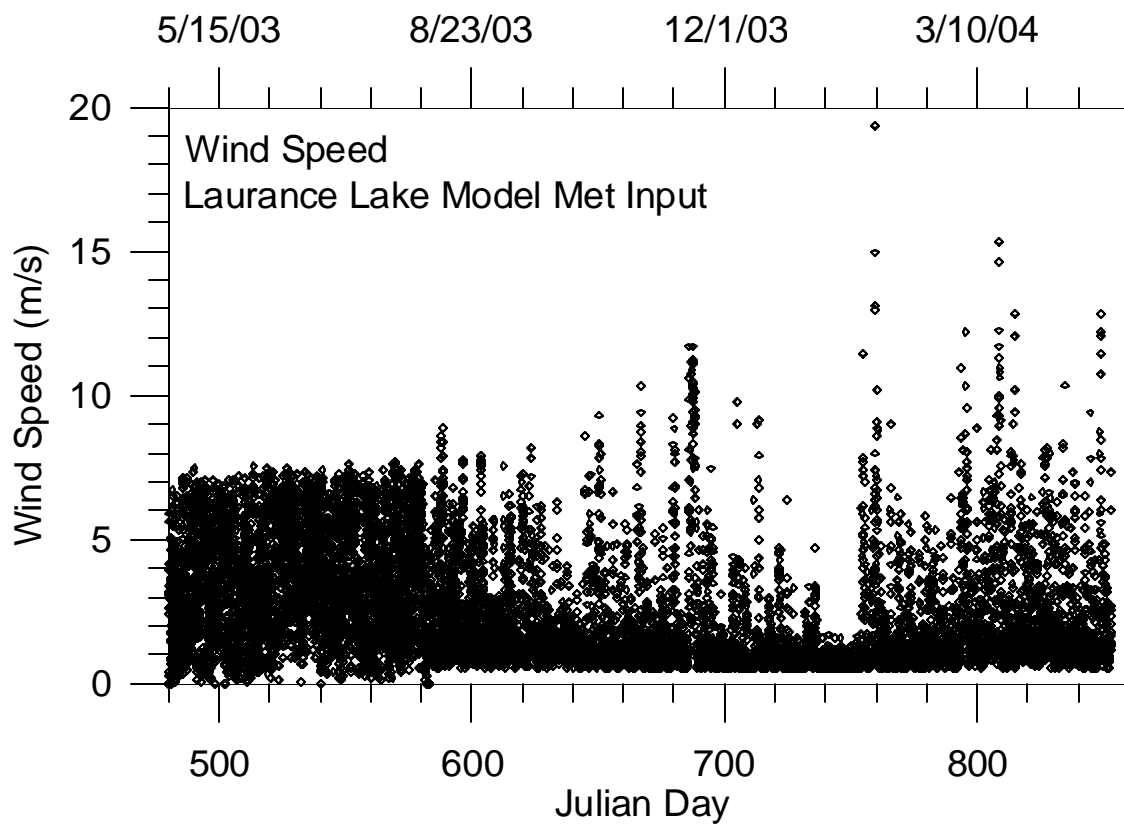


Figure 24. Wind Speed, m/s

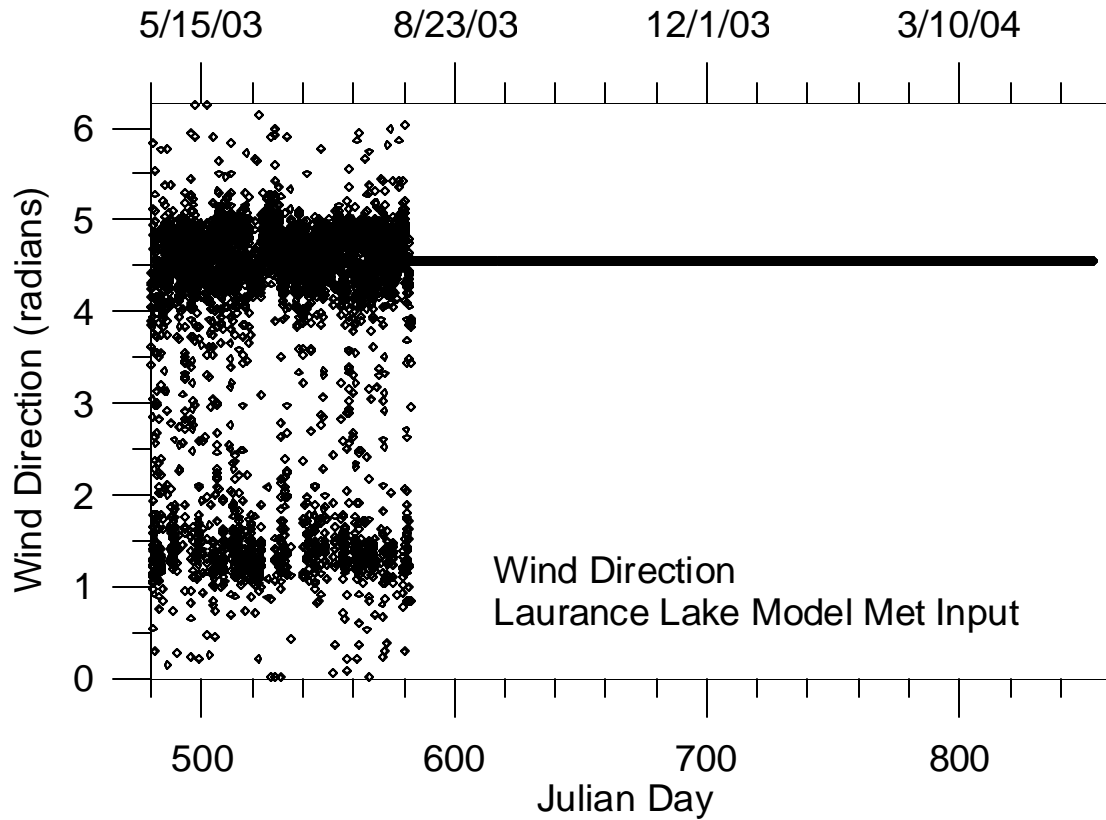


Figure 25. Wind direction used for model input. After Julian Day 583 (8/6/03) wind data measured at the dam did not exist and was set to a value of 4.55 radians, roughly parallel to the axis of the reservoir.

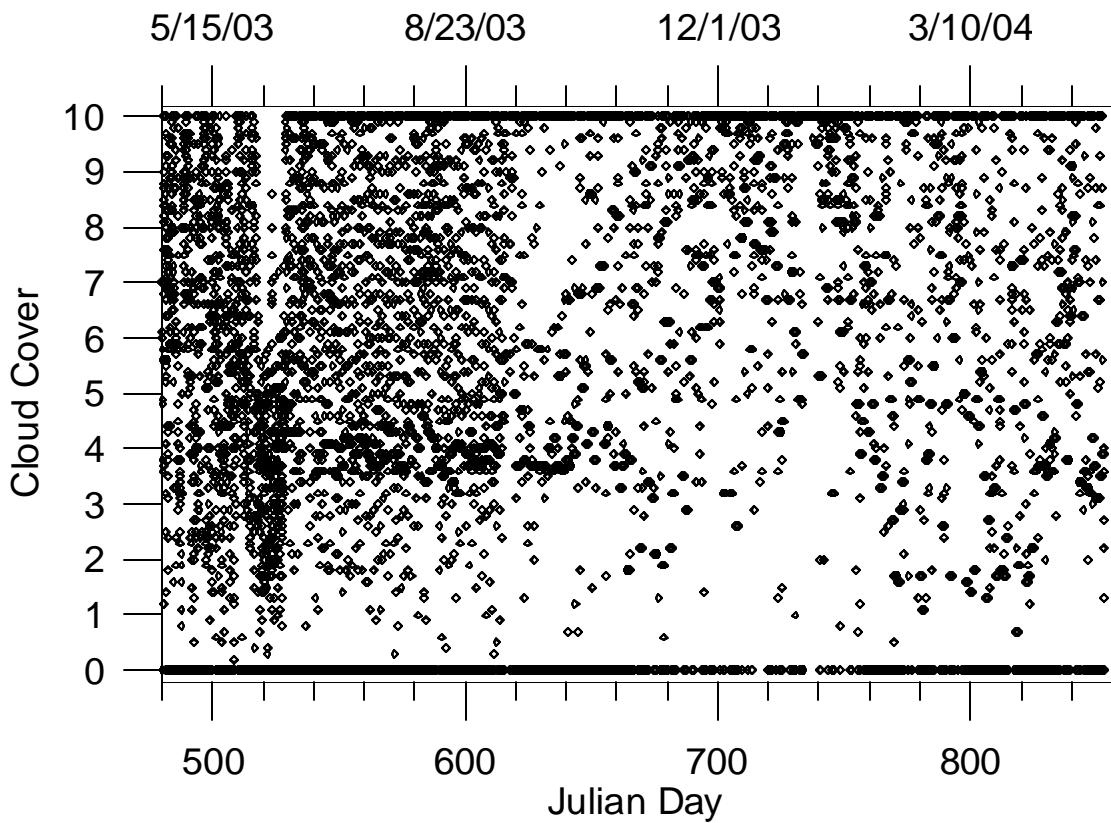


Figure 26. Cloud Cover

Water Year

The calibration period of May 2003 and April 2004 was analyzed in order to determine if it was an average or below average water year. Flow rates measured at the United States Geological Survey gauging station for the Hood River at Tucker Bridge (USGS 14120000) were used. There were 49 years of data available, and it was assumed that the wetness or dryness of a year on Clear Creek above the dam could be determined by evaluating flows at this site.

A frequency plot showing the number of years that were wetter and dryer than the calibration period was shown in Figure 27. For the period of May through April, 18% of the years were dryer and 82% of the years were wetter.

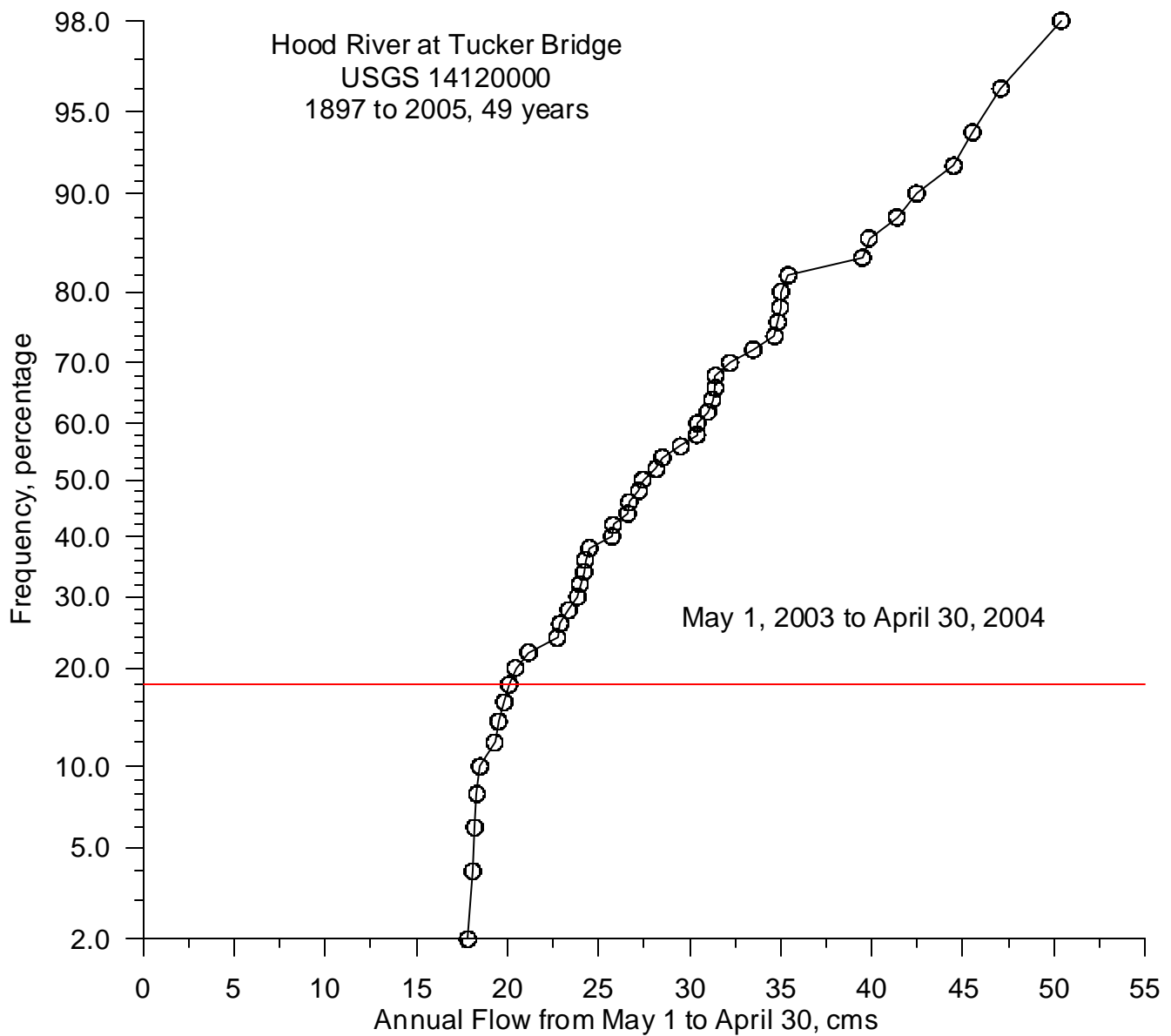


Figure 27. Frequency plot showing the occurrence of years wetter or dryer than the calibration period. 18% of the years were dryer, 82% were wetter.

Detention Time

Figure 28 shows the detention time plotted versus flow rate for different water levels. At a full pool water surface elevation with combined inflows of 20 cfs, the detention time would be approximately 80 days.

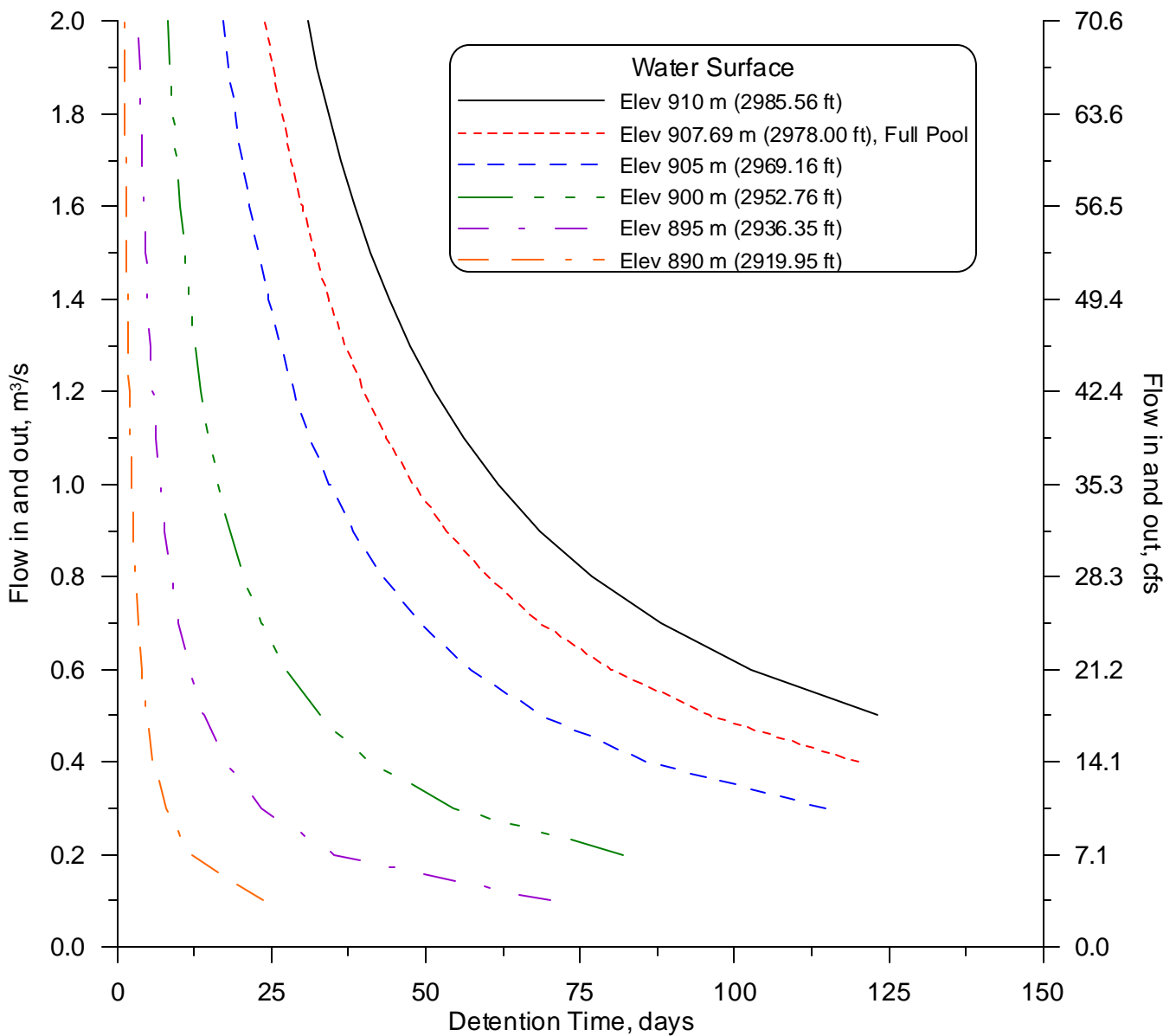


Figure 28. Plot of flow versus detention time for different water level elevations.

Hydrodynamic Calibration

Water level data were compared with model results were shown in Figure 29. Table 2 shows water level statistics. Water levels were calibrated by adding a distributed flow file to compensate for the error in inflow/outflow measurements and to also account for inflows and/or losses directly into the reservoir.

Table 2. Water level error statistics.

n, # of data comparisons	Mean Error M	Absolute Mean Error m	Root Mean Square Error m
365	0.009	0.036	0.078

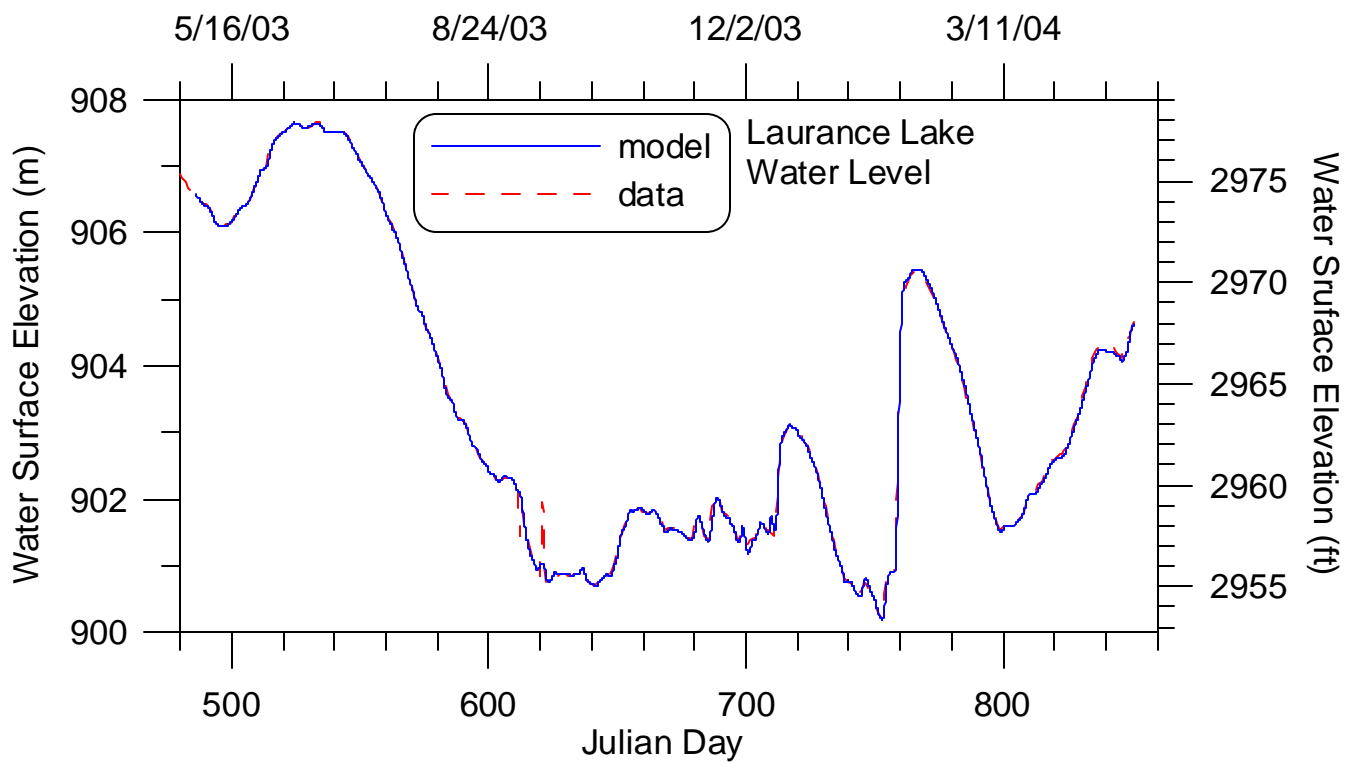


Figure 29. Water level prediction compared with data for Laurance Lake.

Temperature Calibration

Model parameters affecting temperature calibration included wind sheltering coefficients, groundwater inflow temperature, and the accurate representation of reservoir outflows. Temperature predictions in Laurance Lake were particularly sensitive to the wind-sheltering coefficient. When wind data collected at the dam were available, a wind sheltering coefficient of 0.75 was used near the dam and values of 0.60 were used in segments upstream. A time varying light extinction coefficient was used for the entire simulation period. The light extinction coefficient values were based on Secchi disk data, and values ranged from 0.27 to 0.96.

Vertical Profiles

Temperature probes located along the dam were measured continuously over the simulation period. Figure 30 through Figure 33 show the comparison between model predictions and temperatures measured at the dam at 10 day intervals. Table 3 list error statistics between model predictions and data for all the sampling locations in the reservoir.

Table 3. Temperature profile error statistics. ‘RMS’ represents root mean square error and ‘AME’ is absolute mean error.

Site	N, # of data profile comparisons	Temperature model –data error statistics	
		AME, °C	RMS error, °C
Dam	365	0.53	0.59
L1		0.55	0.61
L2		0.58	0.66
L3		0.76	0.84
L4		0.56	0.60

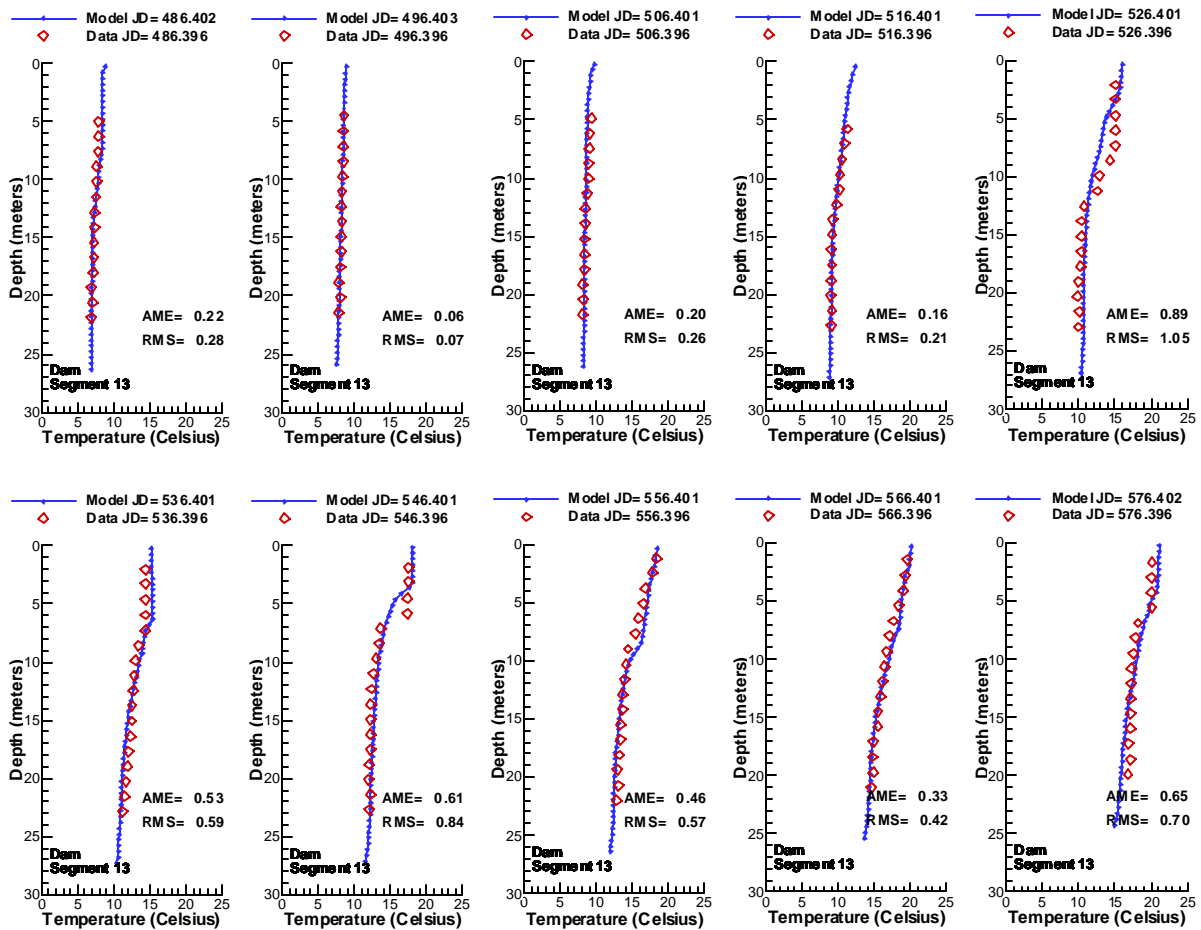


Figure 30. Comparison of model predicted vertical temperature profiles and data collected at dam (Julian Day 486 to Julian Day 586). ‘AME’ is absolute mean error and ‘RMS’ is root mean square error.

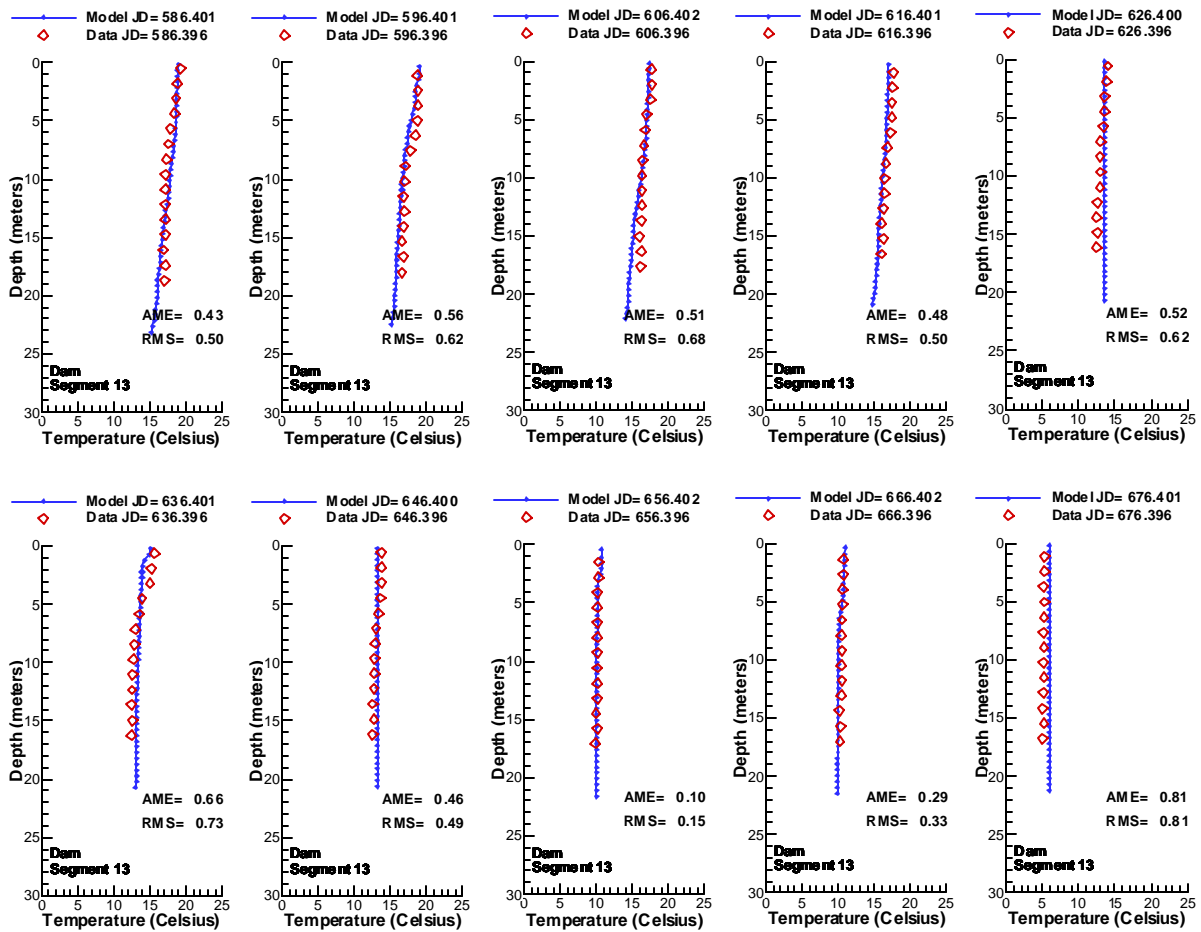


Figure 31 Comparison of model predicted vertical temperature profiles and data collected at dam (Julian Day 586 to Julian Day 676). ‘AME’ is absolute mean error and ‘RMS’ is root mean square error.

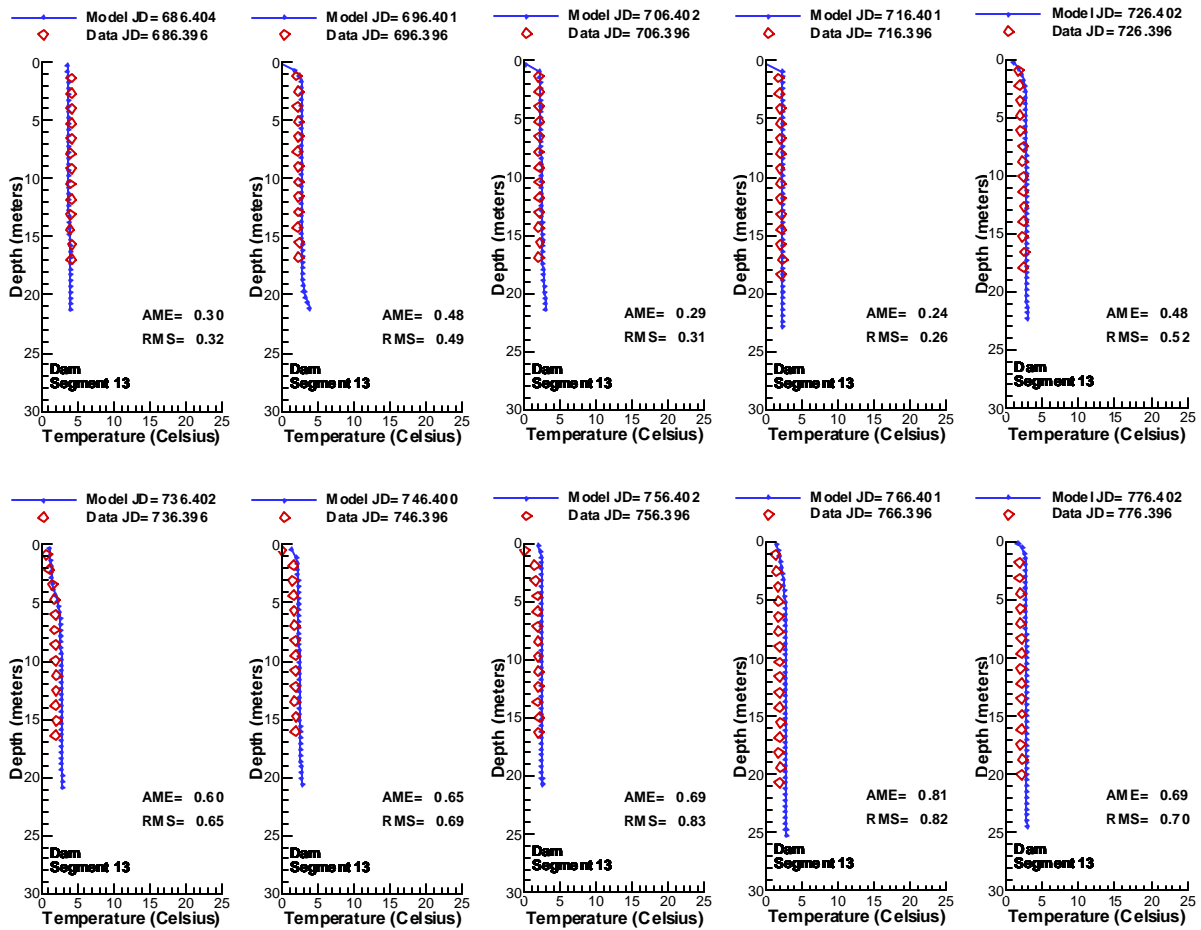


Figure 32 Comparison of model predicted vertical temperature profiles and data collected at dam (Julian Day 686 to Julian Day 776). ‘AME’ is absolute mean error and ‘RMS’ is root mean square error.

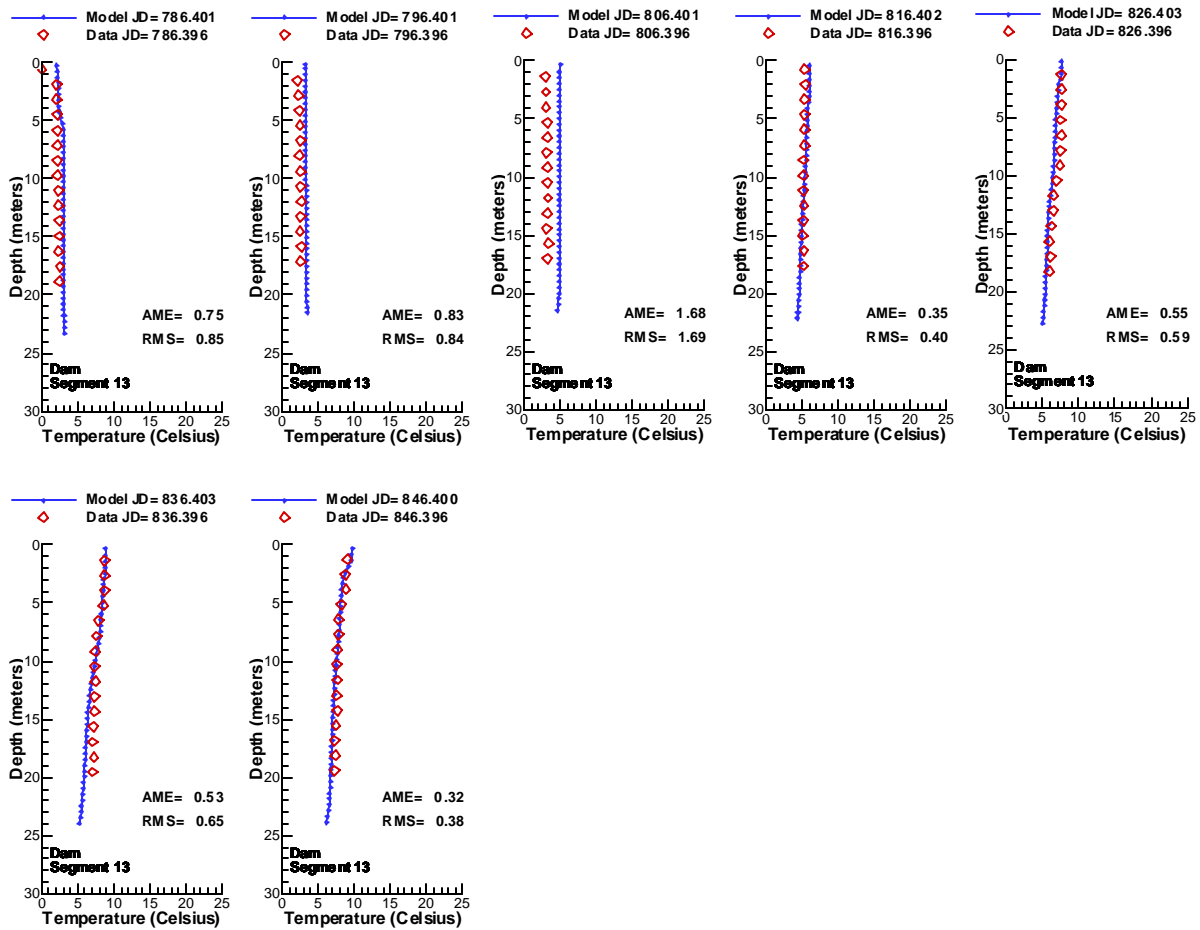


Figure 33. Comparison of model predicted vertical temperature profiles and data collected at dam (Julian Day 786 to Julian Day 846). ‘AME’ is absolute mean error and ‘RMS’ is root mean square error.

Management Scenarios – Part 1

The management scenarios were simulated in two parts: (1) an initial 10 scenarios; (2) and an additional 36 scenarios using varied fish flows, water levels and outlet hardware. This section discusses the initial 10 management scenarios which were described in Table 4. Scenario 1 was the base case and was simply the calibrated model without any changes except that the simulation period is May 1, 2003 through October 31, 2003. The scenarios are all identical to the calibrated model except for characteristics listed below in Table 4.

Table 4. Laurance Lake scenario descriptions.

Scenario #	Name	Description
1	Base Case	Calibrated model, Time period May 1 through October 31, 2003
2	Outlet near water surface	Dam outlet is kept at water surface
3	Outlet near bottom or near water surface, Threshold is water temperature of 15 degrees Celsius	Outlet near water surface if outlet temperature below 15 degrees Celsius. Otherwise outlet moved to bottom
4	Dam raised 12 meters	Water surface raised 12 meters above calibrated simulation
5	Dam raised 2 meters	Water surface raised approximately 2 meters above calibrated simulation
6	Pinnacle Creek Diversion	½ of flow from Pinnacle Creek diverted to Clear Creek below dam
7	Outlet at bottom and surface – option 1	Pass all irrigation and powerhouse flows from the surface outlet, pass 3 cfs to Clear Creek below dam until 9/15, then increase to 15 cfs. On 10/1 increase flow to 30 cfs.
8	Outlet at bottom and surface – option 2	Pass all irrigation and powerhouse flows and fish flows to Clear Creek from the surface outlet, once surface outlet becomes > 10°C, pass Clear Creek flows from lake bottom: 3 cfs to Clear Creek below dam until 9/15, then increase to 15 cfs. On 10/1 increase flow to 30 cfs.
9	Outlet at Surface and outlet at bottom	50% of outflows withdrawn near water surface, 50% withdrawn near bottom
10	Dam raised 12 meters, with outlet near surface	Dam outlet is kept near water surface and water surface raised 12 meters above calibrated simulation

Scenario results were summarized by calculating average outflow temperature, plotting outflow temperature versus time, and plotting temperature the difference (Δ T) between outflow temperature and Clear Creek inflow temperature.

Table 5 lists the average outflow temperature and the average temperature difference between the outflow temperature and Clear Creek inflow temperature for all the scenarios.

Table 5. Laurance Lake scenarios average outflow temperatures and average temperature difference between outflow and Clear Creek inflows. The outflow temperature is the water temperature which would be discharged to Clear Creek below the dam.

Scenario #	# days avg. 7-day max exceed 12 degrees Celsius	Avg. Outflow Temp. (C)	Avg. July – August Outflow Temp. (C)	Avg. Aug. 15 – Oct. 15 Outflow Temp. (C)	Avg. Temp. Difference btw. Outflow and Clear Cr. Inflow (C)	Avg. July-August Temp. Difference btw. Outflow and Clear Cr. Inflow (C)	Avg. Aug. 15-Oct. 15 Temp. Difference btw. Outflow and Clear Cr. Inflow (C)
1	103	12.06	14.32	13.99	4.67	5.69	5.98
2	139	14.43	17.34	14.62	6.52	8.39	6.19
3	112	12.23	14.10	13.96	4.85	5.52	5.97
4	38	10.22	9.52	11.84	2.46	0.78	3.64
5	91	11.93	13.58	14.55	4.54	4.99	6.66
6*	97	11.73	14.03	13.56	4.25	5.38	5.50
7	0	8.32	8.53	9.41	0.86	-0.23	1.34
8	1	8.63	8.53	9.44	1.29	-0.23	1.48
9	36	10.22	10.77	12.08	2.82	2.03	4.15
10	156	15.70	18.41	16.48	7.76	9.52	8.22

*Outflow temperature if diverted Pinnacle Creek Flow is included

The model predicted outflow temperatures of scenarios 1 through 5 were plotted in Figure 34. Model predicted outflow temperatures for scenarios 1, and 6 through 10 were shown in Figure 35. Figure 36 and Figure 37 show the 7-day average of the maximum daily temperature for scenarios 1-5 and scenarios 6-10, respectively. As might be expected, the single outlet near the surface (scenarios #2 and #10) predicted the highest outflow temperatures. Diurnal temperature variations near the water surface were apparent in the predicted fluctuations of outflow temperatures for scenario #2 and #10. These scenarios show how warm the outflow can be if water is withdrawn only at surface. Scenario #3 (outlet at the surface until outflow temperatures reached 15 °C, then outlet moved to bottom) had warmer outlet temperatures than the base case scenario when the withdrawal was near the surface, but cooler temperatures for a period of time after the outlet was shifted to the bottom. After the outlet was moved to the bottom for scenario #3, the outflow temperatures remained cooler than the existing condition for approximately 35 days (until early July), after which outflow temperatures were equivalent to scenario #1. Raising the dam 12 meters scenario (#4) produced cooler temperatures than all the other scenarios until mid-September, after which the outflow temperatures were greater. The increased reservoir volume of the raised dam scenario resulted in the greater storage of heat gained during the summer, and the reservoir cooled slower relative to the other scenarios during the fall. Scenario #5, which raised the dam 2 meters, resulted in outflow temperatures cooler than the base case by 1-3 degrees Celsius until early August, after which outflow temperatures were a few degrees warmer than the base case. The

average outflow temperature for scenario #5 was only 0.1 degrees Celsius cooler than the base case. Diverting half the flow from Pinnacle Creek to below the dam (Scenario #6), bypassing the reservoir, reduced the average outflow temperature by 0.3 degrees Celsius over the base case when the diverted Pinnacle Creek flow is included in the outflow temperature calculation.

There was little difference in the outflow predictions between Scenarios #7 and #8. When considering the water that would be discharged directly to Clear Creek below the dam, these scenarios had considerably cooler outflows than any of the other scenarios. For the July-August time periods outflow temperatures on average were cooler than the Clear Creek inflows. Outflow temperatures finally began increasing during mid-September when the reservoir was significantly drawn down. At no time did the 7 day average of the maximum daily temperature exceed 15 degrees Celsius.

With outflow evenly divided between surface and bottom outlets (Scenario #9), the 7 day average of the maximum daily temperature of outflows to Clear Creek also did not exceed 15 degrees Celsius. Outflow temperatures were within several degrees Celsius of the inflow temperature until early September when cool water at the bottom of the reservoir had been depleted.

The temperature differences between dam outflows and Clear Creek inflows for the scenarios were plotted in Figure 38 and Figure 39. Scenario #1, the base case, showed a maximum temperature difference of 9°C around the beginning of August. The dam raising scenario (#4) predicted the least temperature difference until mid-September, after which the other scenarios predicted smaller temperature differences due to the reservoir cooling more rapidly. Temperature differences predicted by scenario #4 were actually negative for periods in the summer, indicating the water temperatures at the bottom of the reservoir were less than Clear Creek inflows. Scenario #5, raising the dam 2 meters, and scenario #6, the Pinnacle Creek diversion scenario, predicted the next smallest average differences between Clear Creek inflows and dam outflows. The near surface withdrawal scenarios #2 and #10 showed the greatest temperature difference of all the scenarios. Scenarios #7 and #8 which withdrew water from the bottom for Clear Creek fish flows had outflow temperatures close to inflow temperatures up until mid-September.

The model predicted vertical temperature profiles of the scenarios for August 15, 2003 were plotted in Figure 40 and Figure 41. The profiles correspond to the model segment adjacent to the dam. Scenarios with an outlet near the surface that withdrew a large fraction of outflows near the surface (scenarios #2, #7, #8, and #10) predicted the greatest temperature stratification. The 12 meter dam raising scenario (#4) also predicted a large temperature difference between the surface and the bottom despite having only a bottom outlet. The increased depth of this scenario facilitated temperature stratification.

A summary of the scenario results was provided in Table 6.

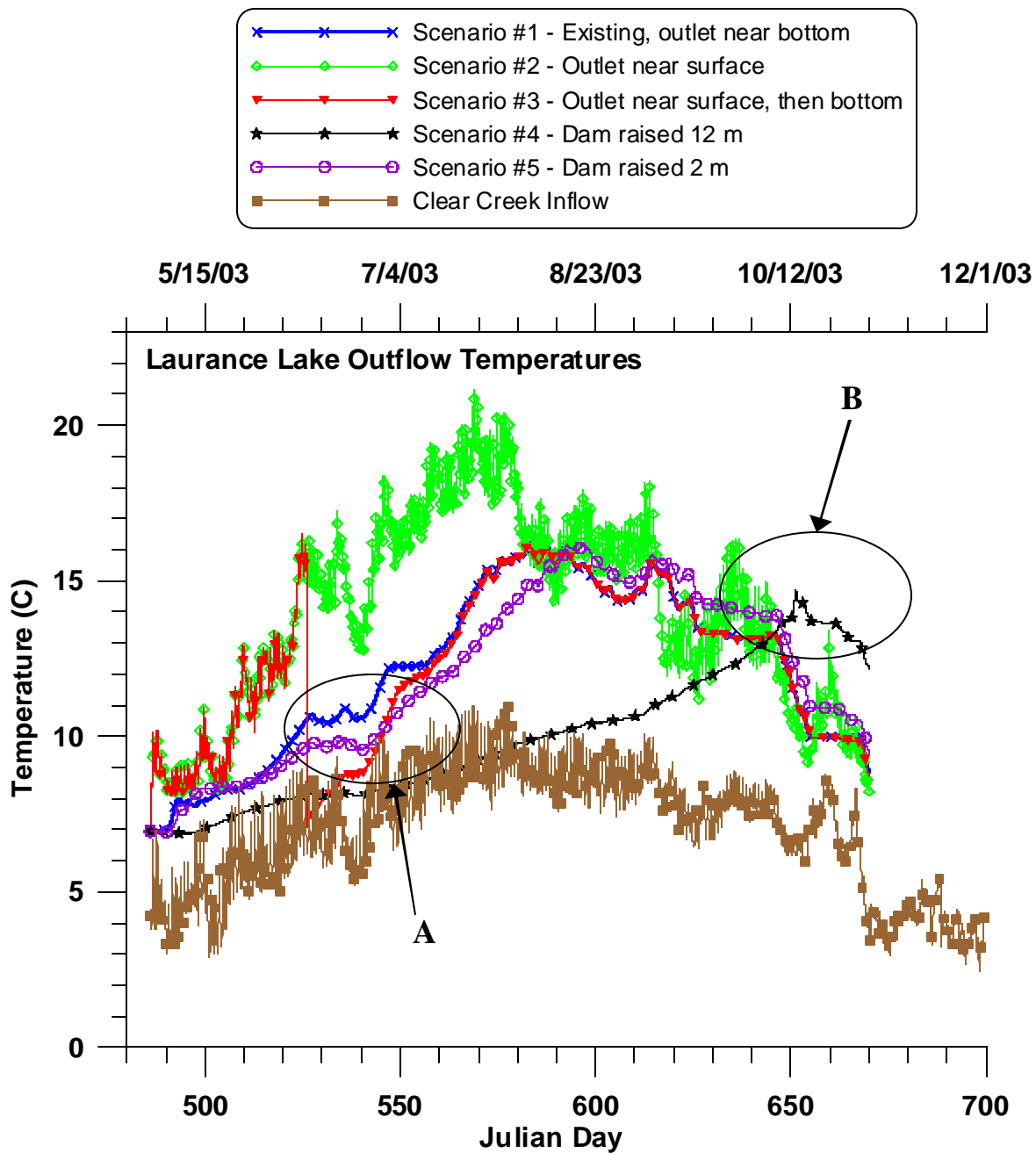


Figure 34 Model predicted outflow temperatures of scenarios 1-5. Item A points out the cool temperature benefit of scenario #3 which lasts for approximately 2 weeks in late June – early July. Item B shows how the raised dam scenario (#4) will predict warmer outflow temperatures beginning in September, even though earlier in the summer the outflow temperatures were cooler than the base case.

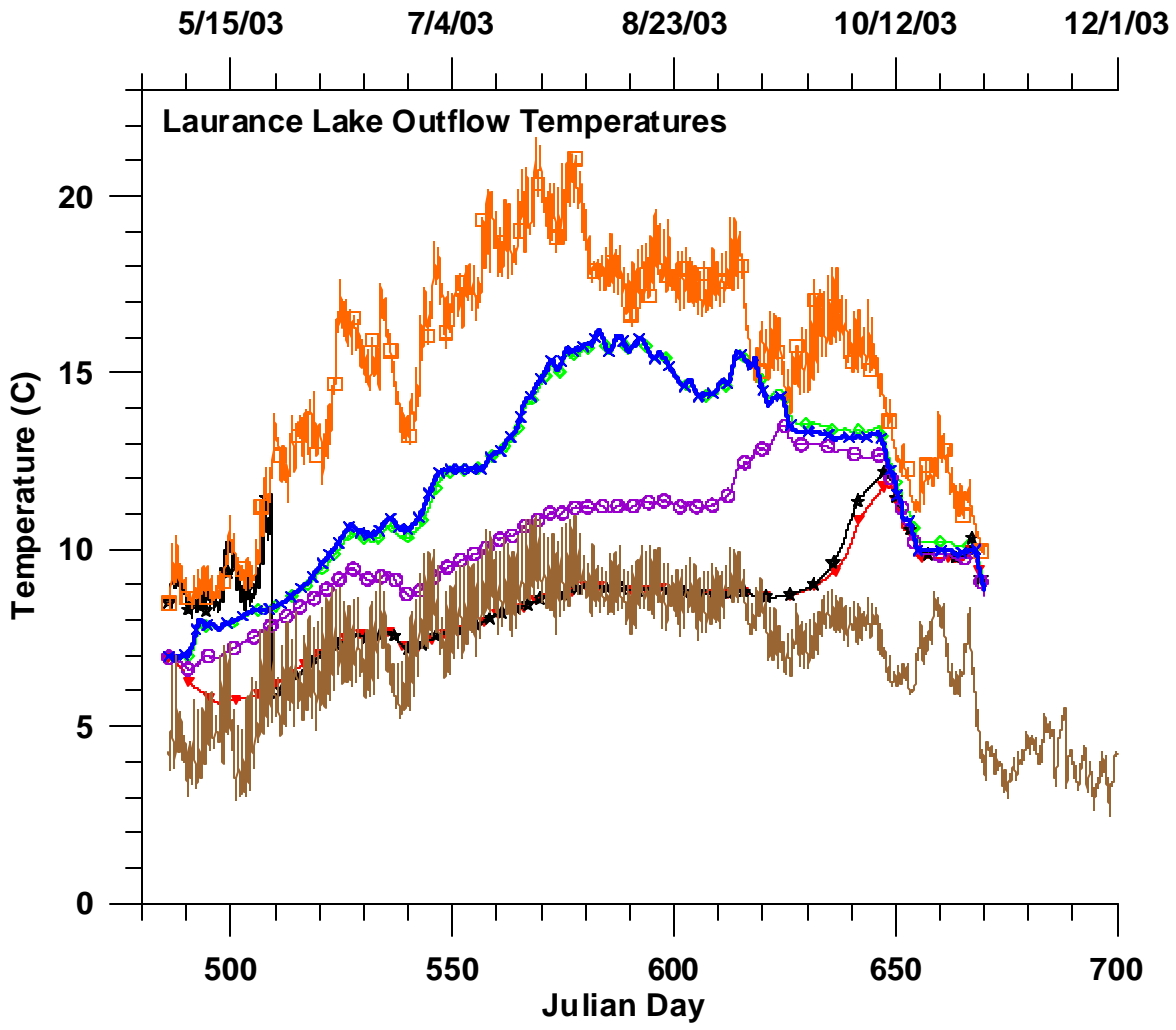
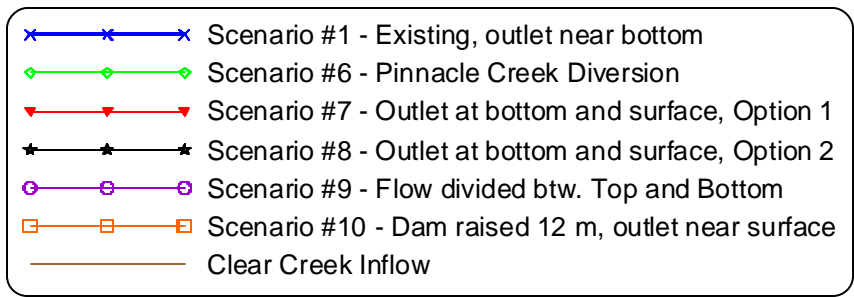


Figure 35 Model predicted outflow temperatures of scenarios 1, 6-10. The outlet temperatures for Scenarios #7, #8 and #9 correspond to the temperatures withdrawn from the bottom outlet only.

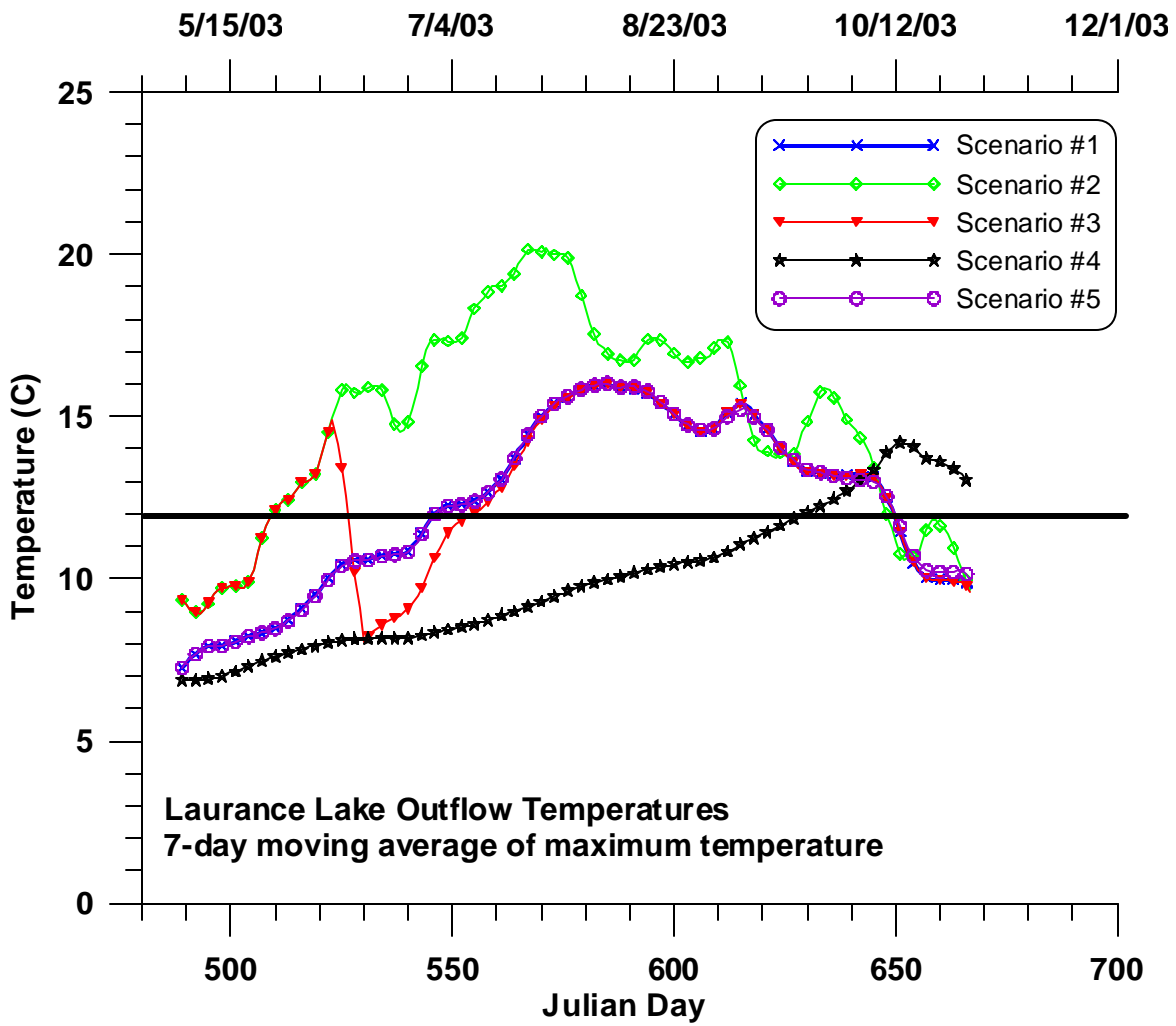


Figure 36. Comparison of 7-day moving average of the daily maximum temperature for scenarios 1-5.

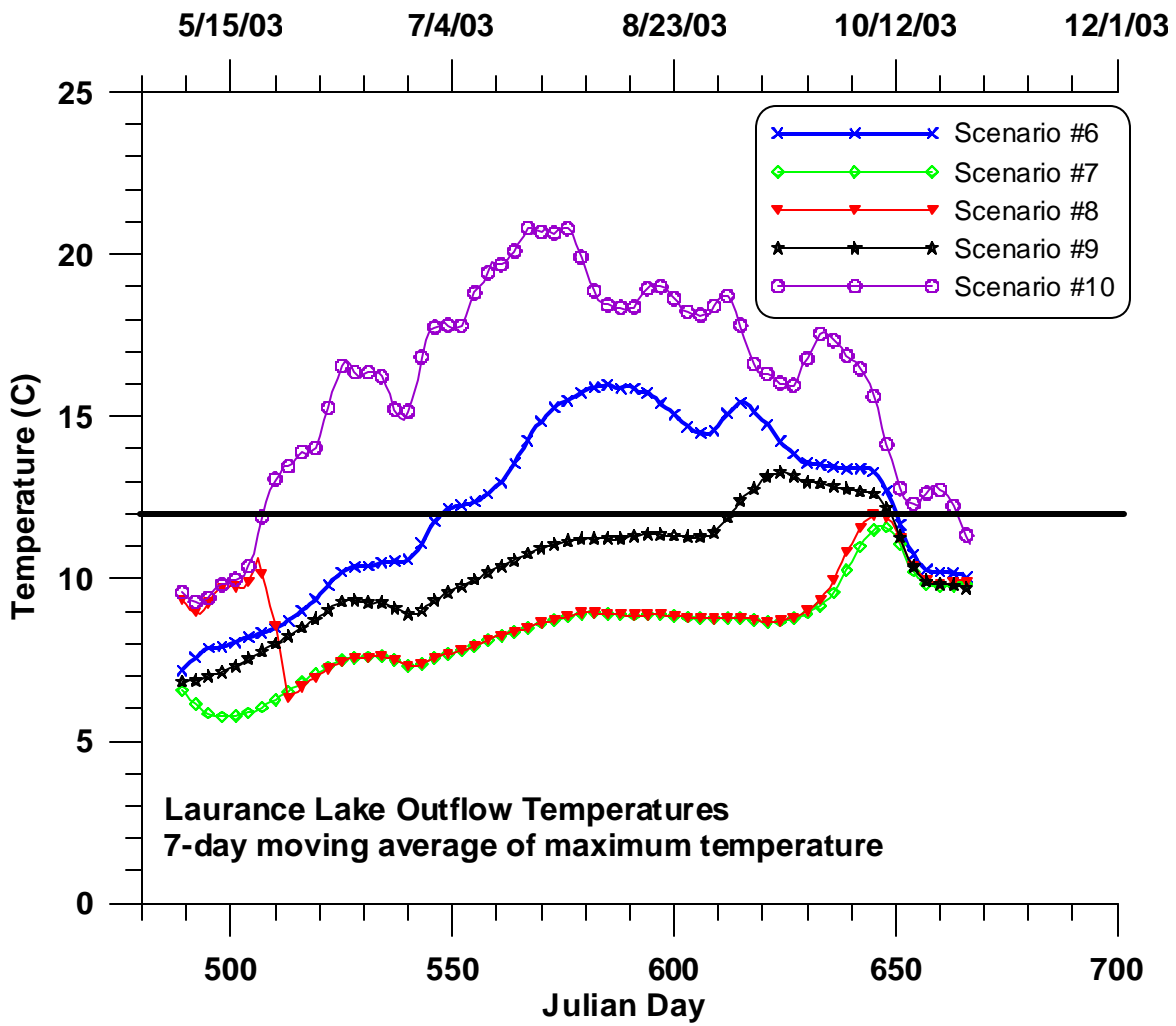


Figure 37. Comparison of 7-day moving average of the daily maximum temperature for scenarios 6-10.

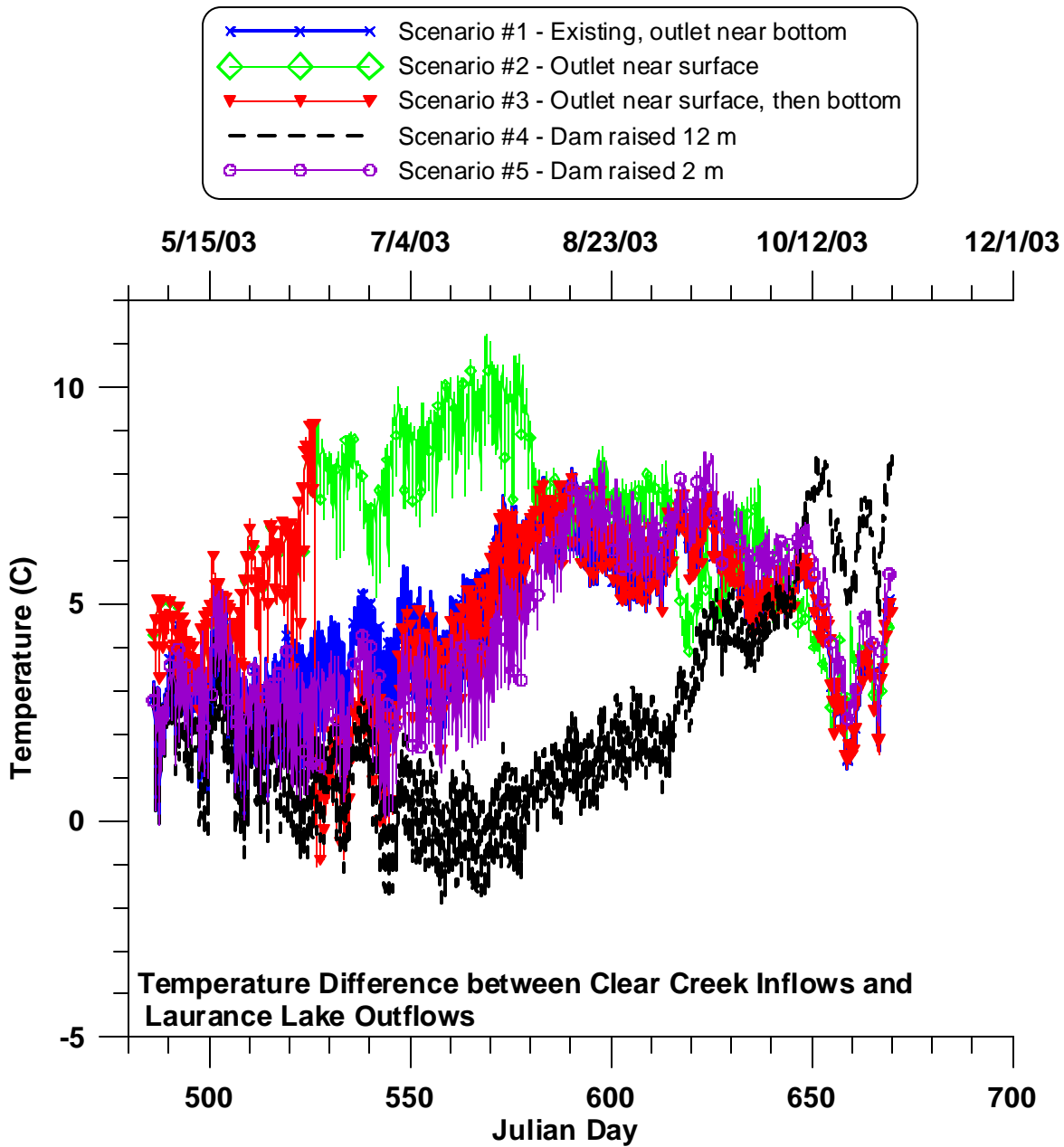


Figure 38 Predicted temperature difference between Clear Creek inflows and dam outflows for scenarios 1-5.

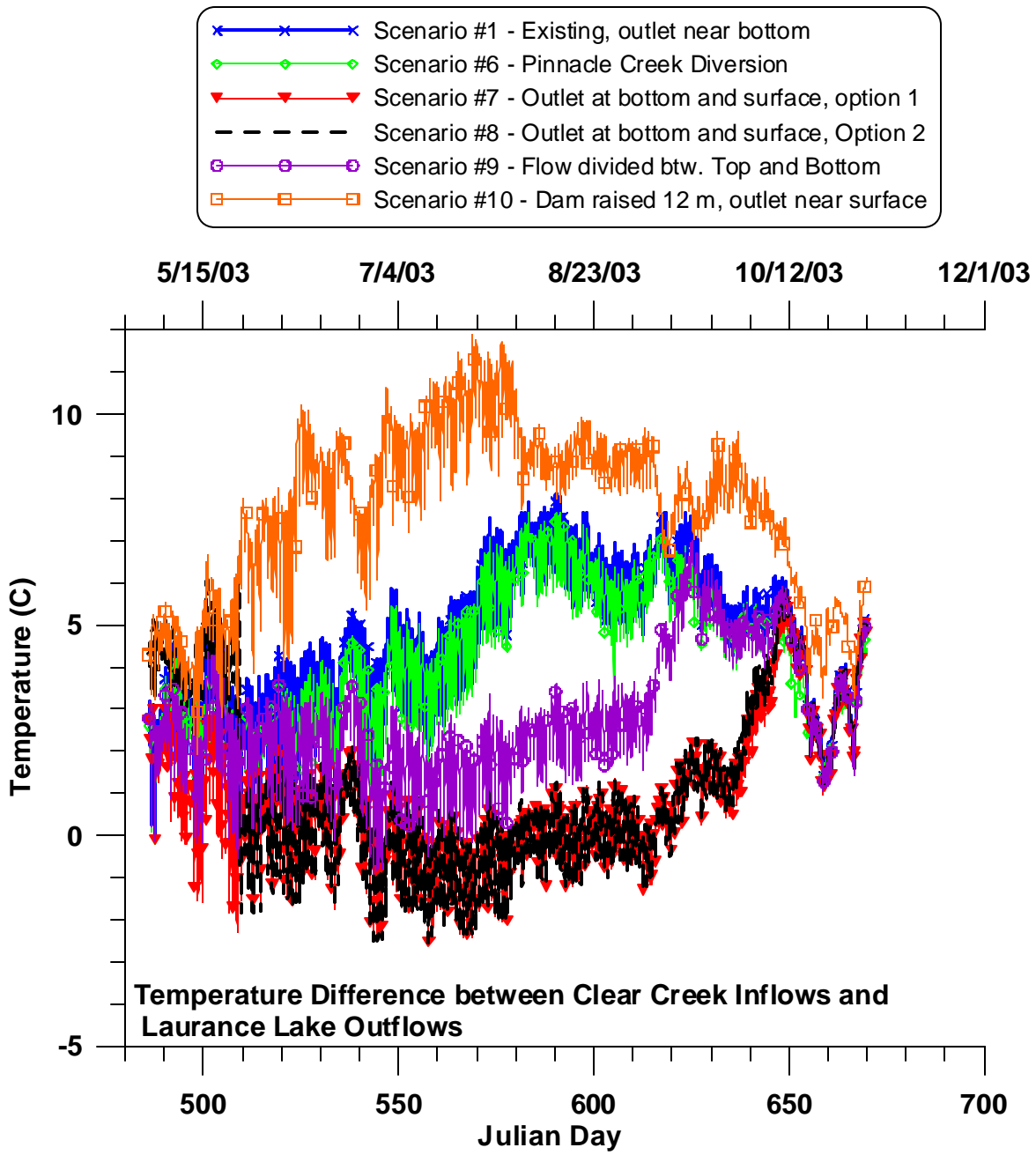


Figure 39 Predicted temperature difference between Clear Creek inflows and dam outflows for scenarios 1, 6-10.

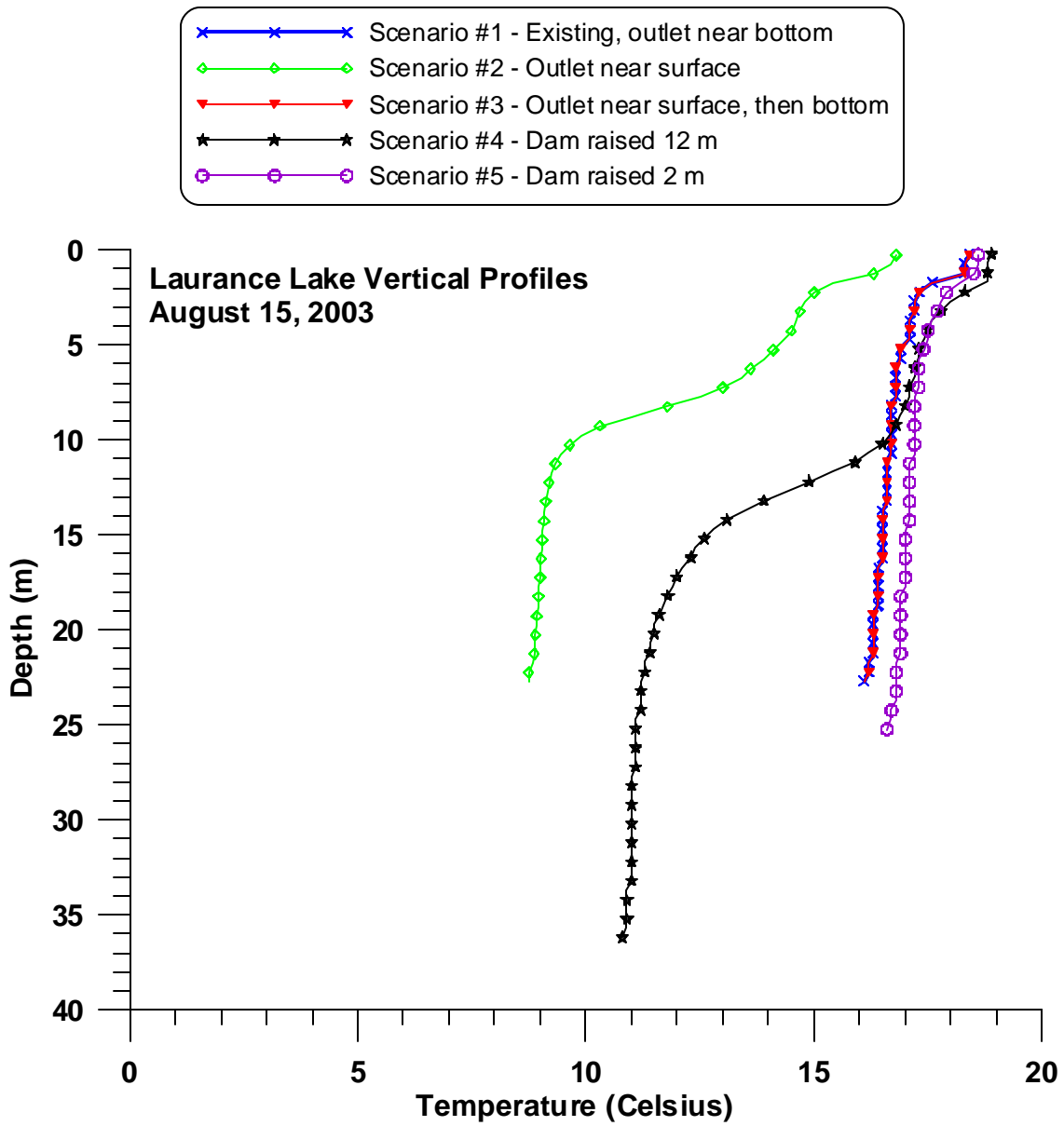


Figure 40 Predicted temperature profile for August 15, 2003 for scenarios 1-5

- ✕ Scenario #1 - Existing, outlet near bottom
- ◇ Scenario #6 - Pinnacle Creek Diversion
- ▼ Scenario #7 - Outlet at bottom and surface, Option 1
- ★ Scenario #8 - Outlet at bottom and surface, Option 2
- ⊖ Scenario #9 - Flow divided btw. Top and Bottom
- Scenario #10 - Dam raised 12 m, outlet near surface

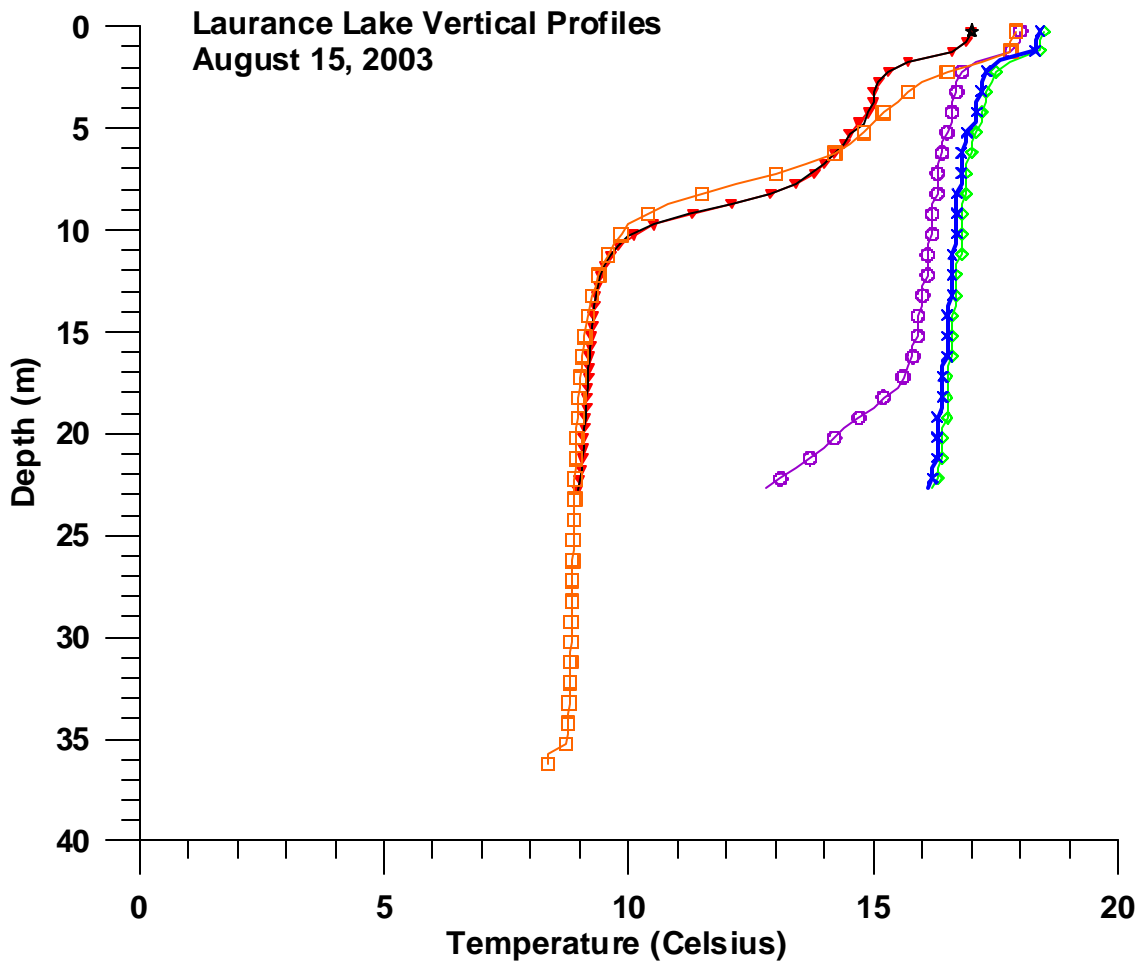


Figure 41 Predicted temperature profile for August 15, 2003 for scenarios 1, 6-10.

Table 6. Summary of scenario results.

Scenario #	Name	Result Summary
1	Base Case	Calibrated model, Time period May 1 through October 31, 2003
2	Outlet near water surface	Warm outflow temperatures
3	Outlet near bottom or near water surface, Threshold is water temperature of 15 degrees Celsius	Improvement for only 2 weeks in late June (see Item A on Figure 34) – early July, but better than base case.
4	Dam raised 12 meters	Cooler for most of the summer, warmer in September-October (see item B in Figure 34)
5	Dam raised 2 meters	Slightly cooler until early August, afterwards slightly warmer
6	Pinnacle Creek Diversion	Slightly improved outflow temperature predictions over entire simulation period
7	Outlet at bottom and surface – option 1	Temperatures for bottom outlet as cool as Clear Creek inflows for most of summer, then warmer starting in September
8	Outlet at bottom and surface – option 2	Very similar results to Scenario 7. Temperatures for bottom outlet as cool as Clear Creek inflows for most of summer, then warmer starting in September
9	Outlet at Surface and outlet at bottom	Generally warmer than base case, but cooler in early August. Maximum outflow temperature couple degrees cooler than base case.
10	Dam raised 12 meters, with outlet near surface	Very warm outflow temperatures

Management Scenarios – Part 2

Scenario Descriptions

The second phase of the management scenarios involved operational changes including altering outflows from the lake for both power/irrigation and fish flows. For example, the minimum flow including seepage from the dam for fish below the dam is approximately 5 cfs. Currently, the flow is increased to 15 cfs on 9/1 and 30 cfs on 9/15 each year. The model was used to explore changing these values as shown in

Table 7.

The modeling was performed to optimize cool temperatures between mid-August and mid-October during the modeling period, the goal being to manage to a 15°C 7-day daily maximum temperature.

Table 7. Current required fish flows below Laurance reservoir and scenario flow range.

Flow period	Existing required flows (including seepage from the dam)	Modeling scenario range
All times	5 cfs	5-10 cfs
After 9/1	15 cfs	5-15 cfs
After 9/15	30 cfs	10-30 cfs

The scenarios listed in Table 8 incorporate differing combinations of fish flows before September 1, from September 1 to September 15, and after September 15. Before 9/1, flow rates of 5, 7.5 and 10 cfs were simulated. From 9/1 to 9/15, flow rates of 5, 10 and 15 cfs were used. After September 15, fish flow rates of 10, 20 and 30 cfs were simulated. For scenarios 11 to 37 water levels in the reservoir were allowed to rise and fall depending on outflows.

Table 8. Flow rates used for fish flows to Clear Creek below dam for Scenarios 11 to 37. Water levels in Laurance Lake were allowed to rise and fall according to outflows.

Scenario #	Fish Flows		
	before 9/1 (cfs)	After 9/1 (cfs)	After 9/15 (cfs)
11	5	5	10
12	5	5	20
13	5	5	30
14	5	10	10
15	5	10	20
16	5	10	30
17	5	15	10
18	5	15	20
19	5	15	30
20	7.5	5	10
21	7.5	5	20
22	7.5	5	30
23	7.5	10	10
24	7.5	10	20
25	7.5	10	30
26	7.5	15	10
27	7.5	15	20
28	7.5	15	30
29	10	5	10
30	10	5	20

Scenario #	Fish Flows		
	before 9/1 (cfs)	After 9/1 (cfs)	After 9/15 (cfs)
31	10	5	30
32	10	10	10
33	10	10	20
34	10	10	30
35	10	15	10
36	10	15	20
37	10	15	30

Additional scenarios with varied fish flows were simulated while keeping the reservoir at full pool. These scenarios were listed in Table 9.

Table 9. Flow rates used for fish flows to Clear Creek below dam for Scenarios 38 to 43. Water level was kept near maximum pool for these scenarios.

Scenario #	Fish Flows		
	before 9/1 (cfs)	After 9/1 (cfs)	After 9/15 (cfs)
38	5	5	10
39	5	10	30
40	5	15	30
41	7.5	15	20
42	10	10	10
43	10	15	30

The final scenarios were described in Table 10. Scenario #44 was identical to scenario 8 but water levels were kept near maximum pool. For this run, fish flows were withdrawn near the surface until outflow temperatures reach 10 degrees Celsius. At that point, fish flows were withdrawn from the bottom of the reservoir while irrigation and powerhouse flows continued to be withdrawn near the surface.

Scenario #45 was identical to scenario #8 except that fish flows were kept at the minimal flow rates of 5 cfs before 9/1, 5 cfs after 9/1 and 10 cfs after 9/15. The goal of this simulation was to preserve cold water at the bottom of the reservoir as far into September-October as possible.

The last scenario, #46, simulated the effect of ramping fish flow increases that begin in September. Rather than increasing fish flow abruptly, from say, 5 cfs to 15 cfs, the flows were increased to the next level incrementally over a 10 day span. The flow rates used for scenario #46 were 5 cfs until 9/15, then an incremental increase to 15 cfs after 9/15, and then an incremental increase to 30 cfs after 10/1.

Table 10. Description of Scenarios #44, #45 and #46.

Scenario #	Name	Description

Scenario #	Name	Description
44	Outlet at bottom and surface, with water levels near maximum pool	Pass all irrigation and powerhouse flows and fish flows to Clear Creek from the surface outlet, once surface outlet becomes > 10°C, pass Clear Creek flows from lake bottom: 3 cfs to Clear Creek below dam until 9/15, then increase to 15 cfs. On 10/1 increase flow to 30 cfs.
45	Outlet at bottom and surface; minimal fish flows	Pass all irrigation and powerhouse flows and fish flows to Clear Creek from the surface outlet, once surface outlet becomes > 10°C, pass Clear Creek flows from lake bottom: 5 cfs to Clear Creek below dam until 9/15, then increase to 10 cfs
46	Existing hardware (outlet at bottom), ramped flow increases	Existing hardware (Outlet at bottom), fish flow increases are “ramped”. Flow rates used for fish flows: 5 cfs to Clear Creek below dam until 9/15, then increase to 15 cfs (over 10 days). On 10/1 increase flow to 30 cfs (over 10 days)

Results

The statistics of outflow temperatures for second phase scenarios were listed in Table 11. The 7-day moving average of the daily maximum temperatures of scenarios 11 to 43 were plotted in Figure 42 to Figure 48. Scenario 44 and scenario 45 were plotted in Figure 49 along with scenarios 1, 8, 32 and 38. Figure 50 shows scenario the 7 day average of the maximum temperature of scenarios 1 and 46.

Existing hardware at the dam was used for scenarios 11 through 43. Outflows passed through the existing bottom outlet. Of the scenarios where water levels in the reservoir were allowed to rise and fall according to demand (scenarios 11 through 37), scenario 32 predicted the coolest outflow temperatures for the August 15 to October 15 period. Outflow temperatures during this period averaged almost 2 degrees cooler than the existing condition. However the July-August outflow temperatures were 0.3 degrees Celsius warmer than the existing condition. Scenario 32 used fish flows of 10 cfs before 9/1, 10 cfs after 9/1 and 10 cfs after 9/15. Temperatures were optimized for August through October because the 10 cfs fish flows before 9/1 allowed more warm water to be released during the summer, and the reduced 10 cfs fish flows after 9/1 kept water levels high enough so that water passing through the bottom outlet was cooler.

Scenarios 38 to 43 simulated varied fish flows while keeping water levels near full pool. The impact of keeping water levels near full pool was cooler outflow temperatures in the summer and warmer outflow temperatures in the fall. Outflow temperatures were approximately 0.2 degrees Celsius warmer than the existing condition for these scenarios during the August 15 to October 15 time period.

Scenario #44 kept water levels near maximum pool with a top and bottom outlet. This simulation was nearly identical to scenario 8 except that the reservoir was kept full. All irrigation, powerhouse flows and fish flows to Clear Creek were passed from the surface outlet until the outflow temperatures exceeded 10°C, afterwards fish flows were passed from the lake bottom at a rate of 3 cfs until 9/15, then

an increase to 15 cfs, and finally an increase on 10/1 to 30 cfs. Outflow temperature were similar to those predicted by scenario 8, except that the cooler outflow temperatures lasted later into the fall, followed by a final warm up which occurred when the cool water at the bottom was finally depleted. Figure 49 compares the 7-day average of the maximum daily temperatures of scenario 44 with scenario 8 and some of the other scenarios.

The scenario predicting the coolest outflow temperatures was scenario #45. The use of minimal fish flows from a bottom outlet and drawing irrigation and powerhouse flows from a surface outlet produced cooler temperatures than any of the other scenarios (Figure 49). This scenario did better than scenario #8 because the reduced fish flows allowed cooler water at the bottom of the reservoir to last longer.

Scenario #46 did slightly better than the existing condition simulation (scenario #1). Ramping the fish flow releases reduced the volume of cool water used for fish flows, allowing this cool water to last longer. The average outflow temperature during the August 15 to October 15 period was decreased by approximately of 0.2 degrees Celsius relative to the existing condition.

Table 11. Scenarios 11 through 46 average outflow temperatures and average temperature difference between outflow and Clear Creek inflows. The outflow temperature is the water temperature which would be discharged to Clear Creek below the dam.

Scenario #	# days avg. 7-day max exceed 12 degrees Celsius	Avg. Outflow Temp. (C)	Avg. July – August Outflow Temp. (C)	Avg. Aug. 15 – Oct. 15 Outflow Temp. (C)	Avg. Temp. Difference btw. Outflow and Clear Cr. Inflow (C)	Avg. July-August Temp. Difference btw. Outflow and Clear Cr. Inflow (C)	Avg. Aug. 15-Oct. 15 Temp. Difference btw. Outflow and Clear Cr. Inflow (C)
11	106	12.10	14.34	14.01	4.77	5.71	6.07
12	103	12.04	14.34	13.88	4.67	5.71	5.92
13	103	12.01	14.34	13.89	4.66	5.71	6.01
14	105	12.07	14.34	13.92	4.74	5.71	5.99
15	103	12.03	14.34	13.95	4.68	5.71	5.97
16	103	11.98	14.34	13.85	4.61	5.71	5.94
17	104	12.08	14.34	13.97	4.72	5.71	5.98
18	103	12.06	14.34	14.03	4.68	5.71	6.01
19	103	12.06	14.34	14.01	4.67	5.71	6.00
20	105	12.02	14.54	13.59	4.67	5.94	5.64
21	101	11.95	14.54	13.33	4.58	5.94	5.47
22	102	11.91	14.54	13.30	4.50	5.94	5.52
23	94	11.99	14.54	13.33	4.60	5.94	5.44
24	103	11.91	14.54	13.32	4.55	5.94	5.47
25	89	11.87	14.54	13.08	4.40	5.94	5.29
26	83	11.85	14.54	12.81	4.44	5.94	5.03
27	84	11.64	14.54	12.39	4.27	5.94	4.84
28	95	11.85	14.54	13.09	4.34	5.94	5.22
29	82	11.65	14.63	12.21	4.29	6.02	4.46

Scenario #	# days avg. 7-day max exceed 12 degrees Celsius	Avg. Outflow Temp. (C)	Avg. July – August Outflow Temp. (C)	Avg. Aug. 15 – Oct. 15 Outflow Temp. (C)	Avg. Temp. Difference btw. Outflow and Clear Cr. Inflow (C)	Avg. July-August Temp. Difference btw. Outflow and Clear Cr. Inflow (C)	Avg. Aug. 15-Oct. 15 Temp. Difference btw. Outflow and Clear Cr. Inflow (C)
30	82	11.64	14.63	12.43	4.29	6.02	4.67
31	92	11.73	14.63	12.73	4.22	6.02	4.81
32	81	11.59	14.63	12.03	4.21	6.02	4.37
33	81	11.63	14.63	12.45	4.24	6.02	4.64
34	83	11.78	14.63	12.60	4.21	6.02	4.49
35	80	11.59	14.63	12.11	4.23	6.02	4.47
36	80	11.63	14.63	12.60	4.27	6.02	4.71
37	80	11.77	14.63	12.48	4.19	6.02	4.22
38	99	12.24	12.16	14.20	4.40	3.38	6.09
39	95	12.17	12.16	14.20	4.38	3.38	6.12
40	95	12.19	12.16	14.20	4.39	3.38	6.13
41	99	12.27	12.18	14.24	4.42	3.41	6.13
42	99	12.27	12.20	14.23	4.42	3.43	6.12
43	95	12.21	12.20	14.22	4.40	3.43	6.15
44	5	8.98	8.57	9.23	1.33	-0.20	1.04
45	0	8.59	8.63	9.17	1.21	-0.13	1.13
46	102	11.96	14.34	13.76	4.59	5.71	5.92

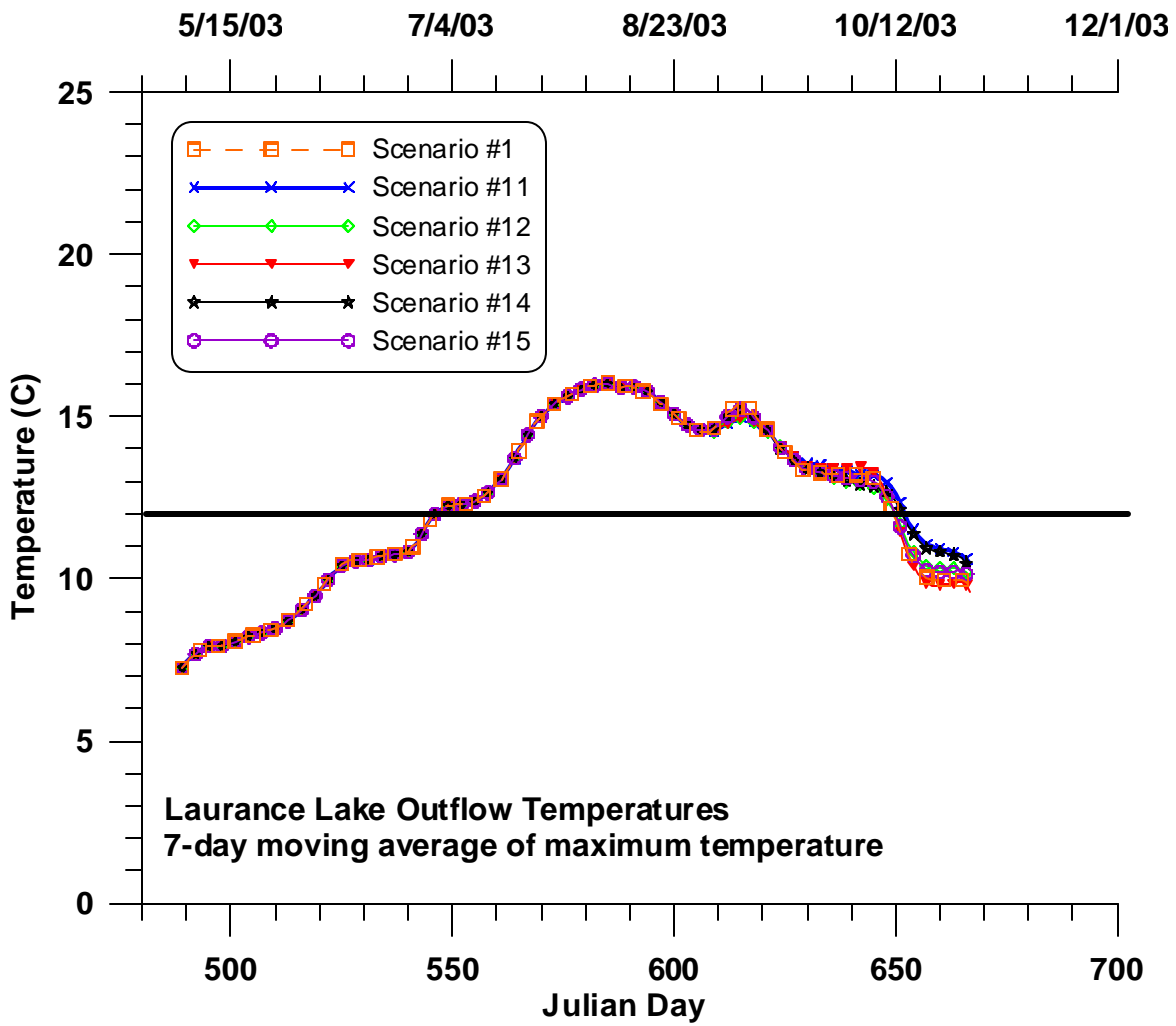


Figure 42. Comparison of 7-day moving average of the daily maximum temperature for scenarios 11-15.

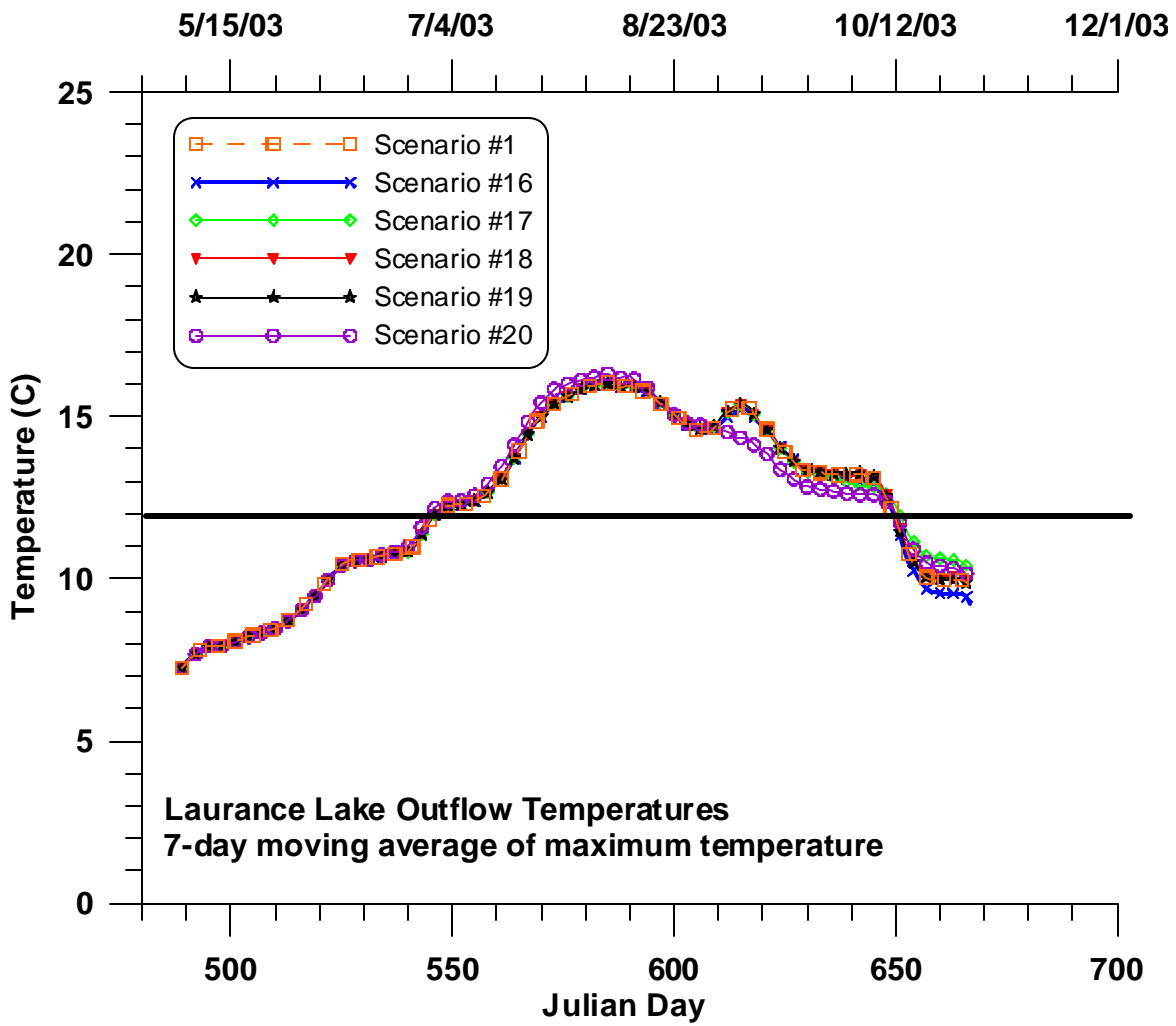


Figure 43. Comparison of 7-day moving average of the daily maximum temperature for scenarios 16-20.

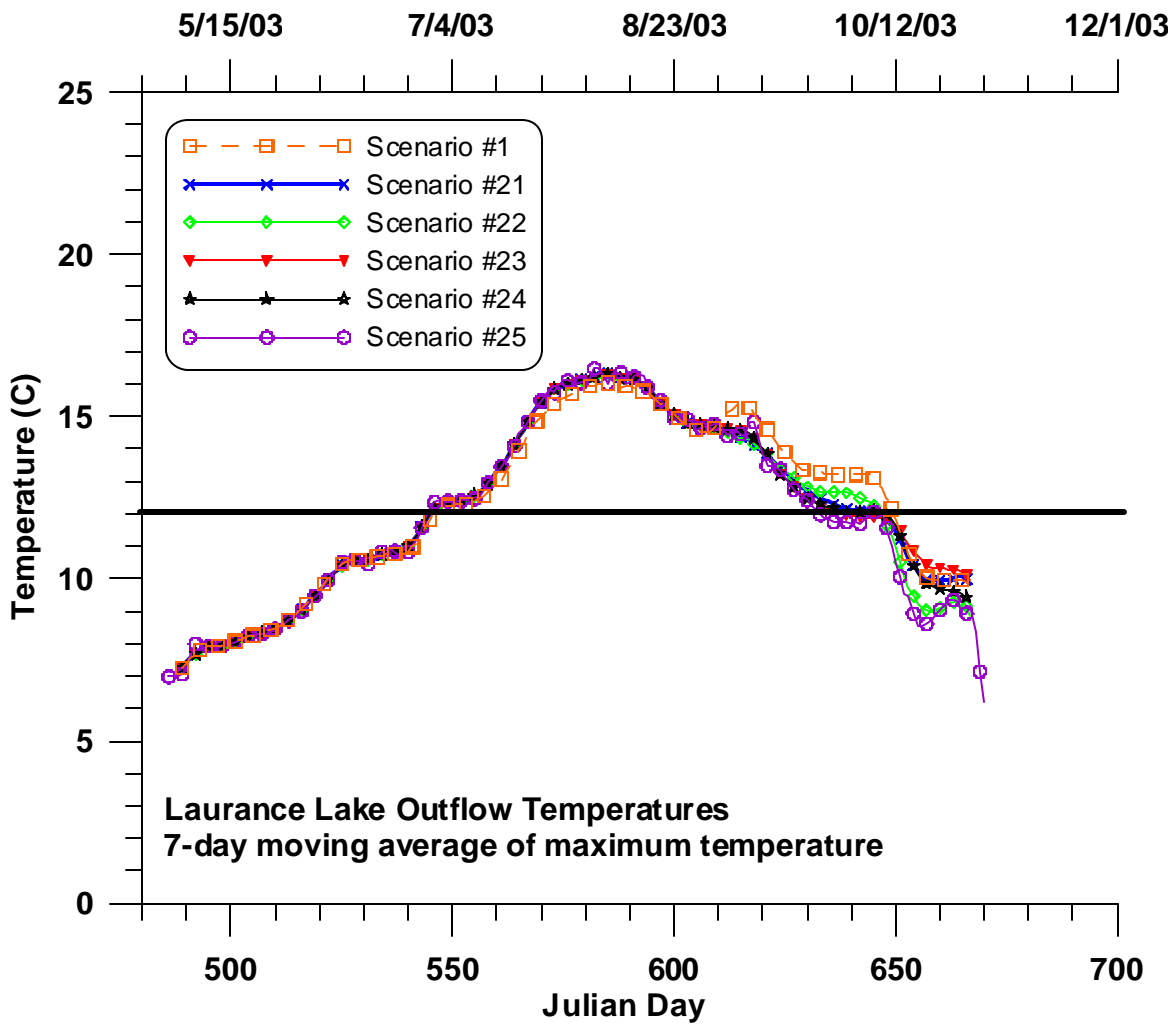


Figure 44. Comparison of 7-day moving average of the daily maximum temperature for scenarios 21-25.

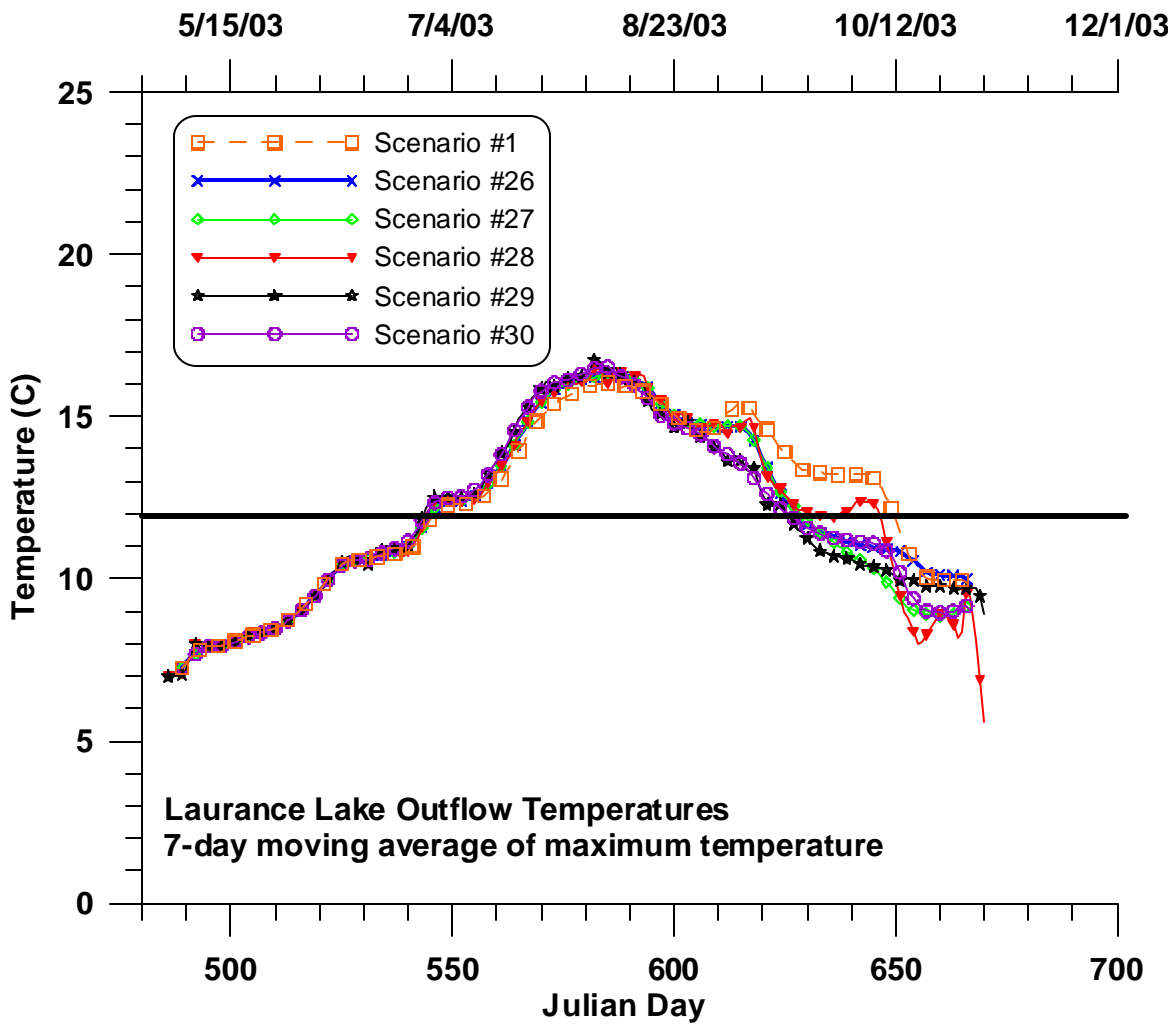


Figure 45. Comparison of 7-day moving average of the daily maximum temperature for scenarios 26-30.

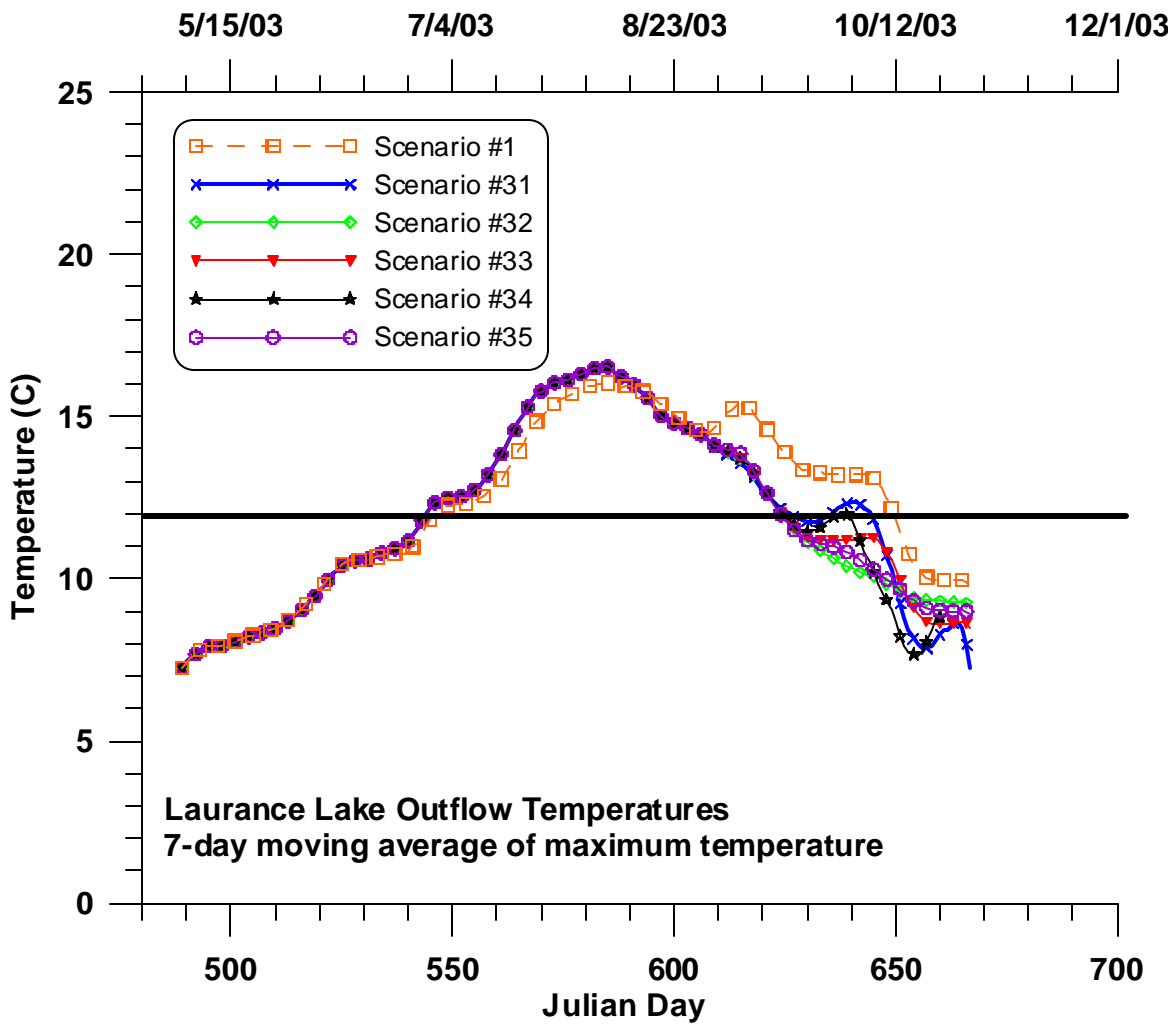


Figure 46. Comparison of 7-day moving average of the daily maximum temperature for scenarios 31-35.

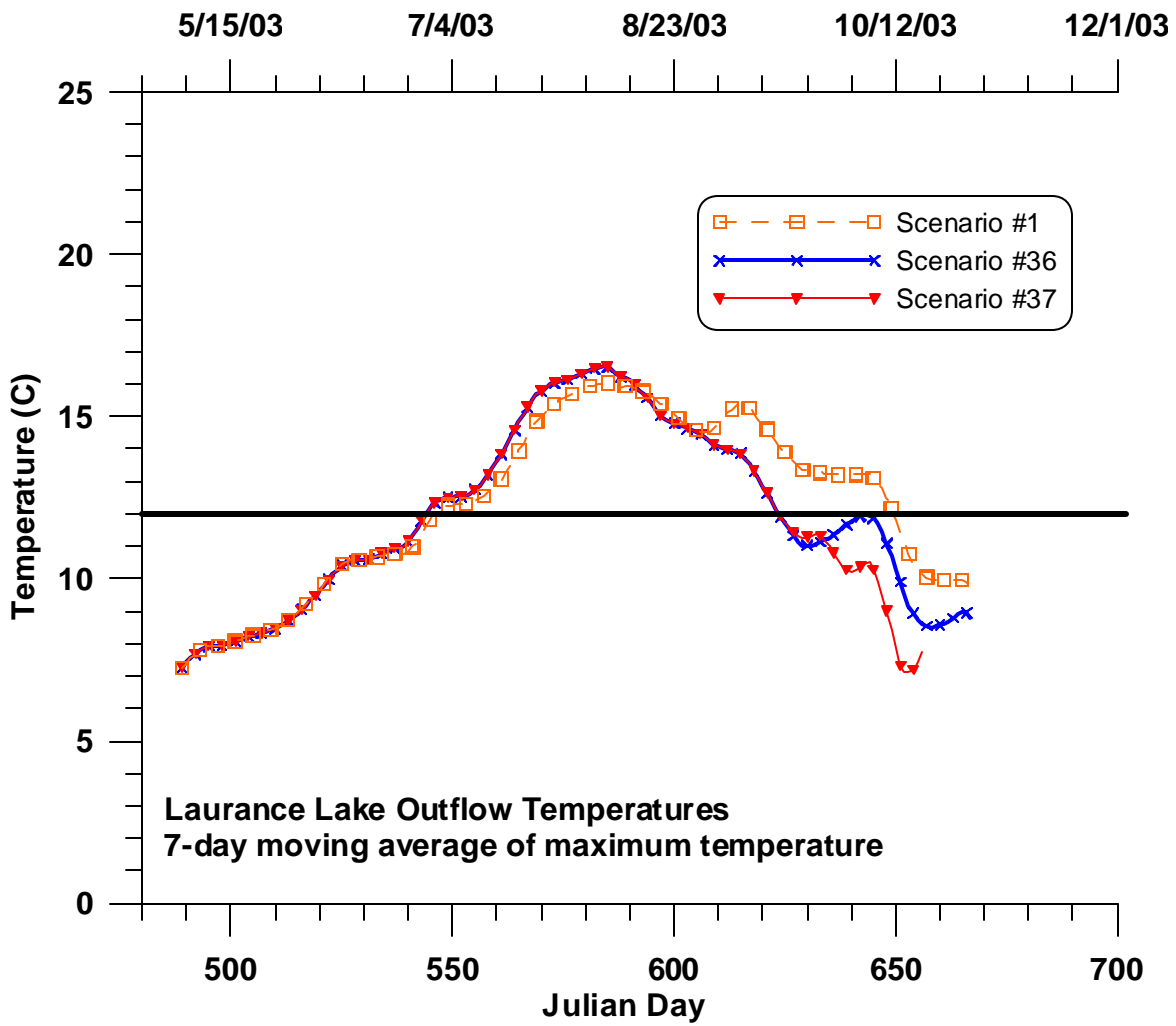


Figure 47. Comparison of 7-day moving average of the daily maximum temperature for scenarios 36-37.

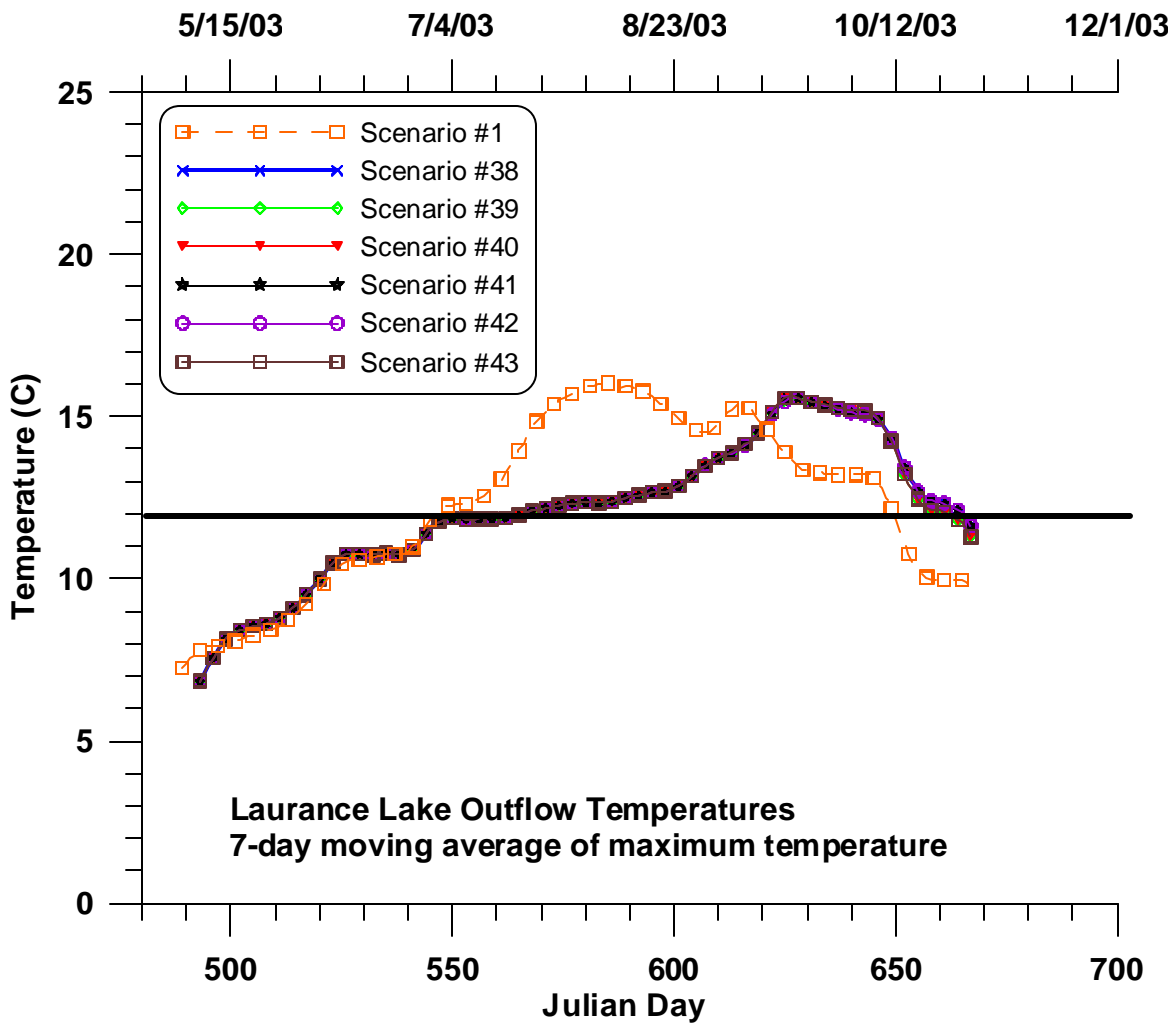


Figure 48. Comparison of 7-day moving average of the daily maximum temperature for scenarios 38-43.

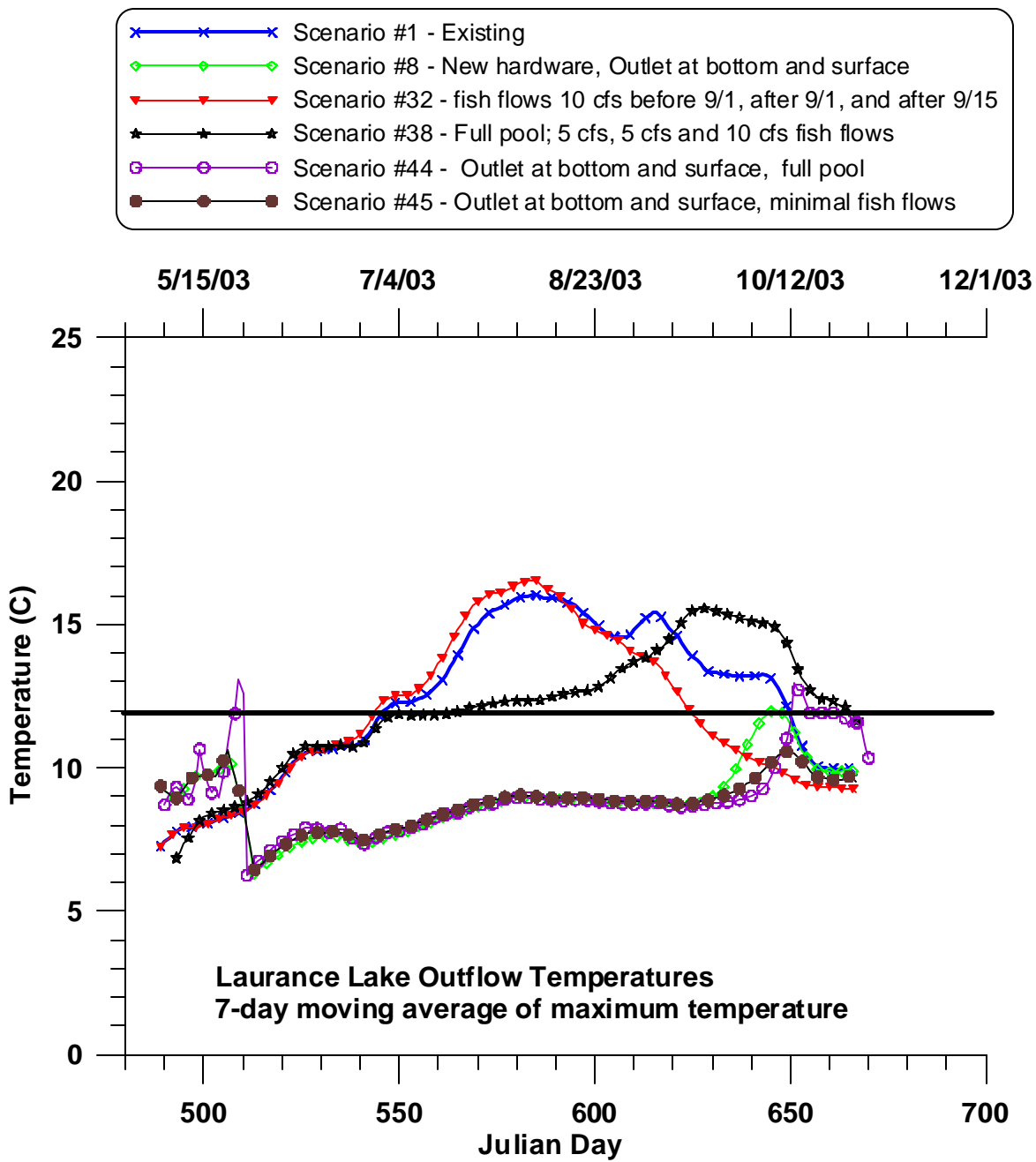


Figure 49. Comparison of 7-day moving average of the daily maximum temperature for scenario #1, #8, #32, #38, #44 and #45.

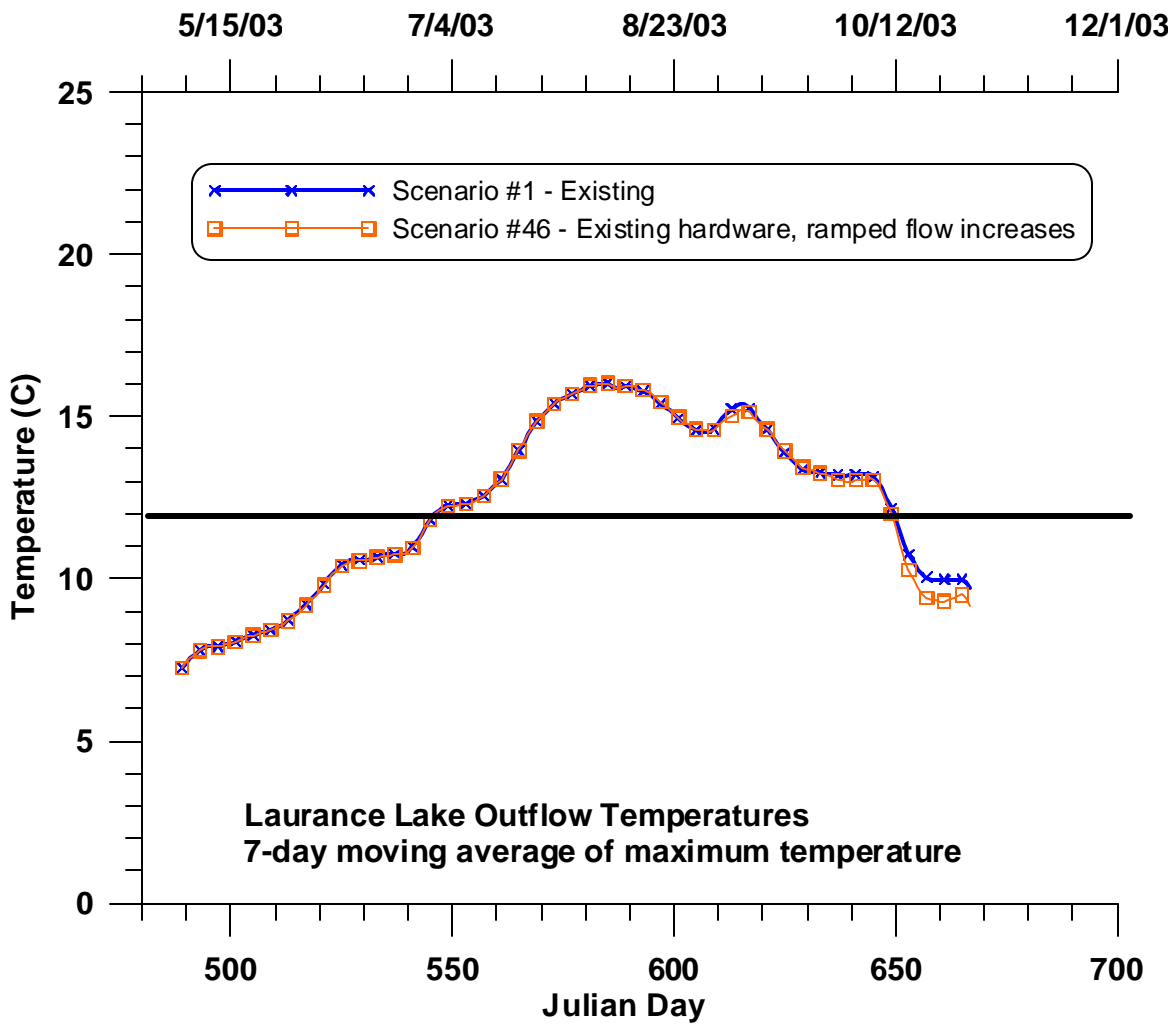


Figure 50. Comparison of 7-day moving average of the daily maximum temperature for scenario 1 and scenario 46.

Table 12. Statistics of selected scenarios 1, 8, 32, 38 and 44.

Scen ario #	# days avg. 7-day max exceed 12 degrees Celsius	Avg. Outflow Temp. (C)	Avg. July – August Outflow Temp. (C)	Avg. Aug. 15 – Oct. 15 Outflow Temp. (C)	Avg. Temp. Difference btw. Outflow and Clear Cr. Inflow (C)	Avg. July-August Temp. Difference btw. Outflow and Clear Cr. Inflow (C)	Avg. Aug. 15-Oct. 15 Temp. Difference btw. Outflow and Clear Cr. Inflow (C)
1	103	12.06	14.32	13.99	4.67	5.69	5.98
8	1	8.63	8.53	9.44	1.29	-0.23	1.48
32	81	11.59	14.63	12.03	4.21	6.02	4.37
38	99	12.24	12.16	14.20	4.40	3.38	6.09
44	5	8.98	8.57	9.23	1.33	-0.20	1.04
45	0	8.59	8.63	9.17	1.21	-0.13	1.13
46	102	11.96	14.34	13.76	4.59	5.71	5.92

Summary

A water quality and hydrodynamic model, CE-QUAL-W2 Version 3.2 (Cole and Wells, 2001; <http://www.cee.pdx.edu/w2>), was applied to Laurance Lake, Oregon. This report summarizes model development and calibration for the CE-QUAL-W2 Version 3.2 model of Laurance Lake.

The system model required that boundary conditions and the topography be determined. Data in support of this modeling effort were shown in this report. This includes data such as:

- Dynamic inflow/discharge rates
- Dynamic inflow/discharge temperatures
- Dynamic inflow/discharge water quality constituents
- Dynamic meteorological data (air temperature, dew point temperature, wind speed, wind direction and cloud cover or short wave solar radiation)
- Model bathymetry

In general, the model reproduces the reservoir responses to the known boundary conditions.

Model scenarios were performed in order to understand the outlet temperature response of the reservoir system. There were 46 scenarios simulated including the following:

- Outlet near water surface (Dam outlet is kept at water surface)
- Outlet near bottom or near water surface, threshold is water temperature of 15°C, Outlet near water surface if outlet temperature below 15°C, otherwise outlet moved to bottom
- Dam raised 12 meters (deep outlet)
- Dam raised 2 meters (deep outlet)
- Pinnacle Creek Diversion where half of the flow from Pinnacle Creek is diverted to Clear Creek below dam
- Outlet at bottom and surface – option 1: Pass all irrigation and powerhouse flows from the surface outlet, pass 3 cfs to Clear Creek below dam until 9/15, then increase to 15 cfs. On 10/1 increase flow to 30 cfs.
- Outlet at bottom and surface – option 2: Pass all irrigation and powerhouse flows and fish flows to Clear Creek from the surface outlet, once surface outlet becomes > 10°C, pass Clear Creek flows from lake bottom: 3 cfs to Clear Creek below dam until 9/15, then increase to 15 cfs. On 10/1 increase flow to 30 cfs.
- Outlet at bottom and surface – option 3: 50% of outflows withdrawn near water surface, 50% withdrawn near bottom
- Dam raised 12 meters, with outlet near surface: Dam outlet is kept near water surface and water surface raised 12 meters above calibrated simulation
- Varying fish flows to Clear Creek
- Varying fish flows to Clear Creek while keeping water levels near full pool
- Outlet at bottom and surface while keeping water levels near full pool: Pass all irrigation and powerhouse flows and fish flows to Clear Creek from the surface outlet, once surface outlet becomes > 10°C, pass Clear Creek flows from lake bottom: 3 cfs to Clear Creek below dam until 9/15, then increase to 15 cfs. On 10/1 increase flow to 30 cfs.

- Outlets at bottom and surface; fish flows from bottom outlet, irrigation and powerhouse flows from surface outlet; fish flows kept at minimal flow rate to allow cool water at bottom of reservoir to last longer
- Ramping the September fish flow increases

Withdrawing water from the surface, then drawing water from the bottom, did lower outlet temperatures over the existing or base case simulation (a lower level outlet). The largest benefit for meeting downstream temperatures during the hot summer months was to withdraw irrigation and powerhouse flows from the surface and withdraw the fish flows for discharge to Clear Creek from the bottom (Scenarios 7, 8, 44 and 45). Keeping water levels near full pool resulted in cooler outflow temperatures in the summer but warmer outflow temperatures in the fall. Reducing fish flows in the fall allowed the volume of cool water at the bottom of the reservoir to last longer into the fall. Scenario #45, which used minimal fish flows along with surface and bottom outlets, predicted the coolest outflows temperatures of any of the scenarios. Figure 49 plots the 7-day moving average of the maximum daily temperature for the some of the more successful scenarios. Figure 51 shows the model predicted temperature difference between Clear Creek inflows and dam outflows for scenarios 1, 8, and 45.

If scenarios such as a lower level outlet are pursued, there should be exploration of the dissolved oxygen impact of these releases.

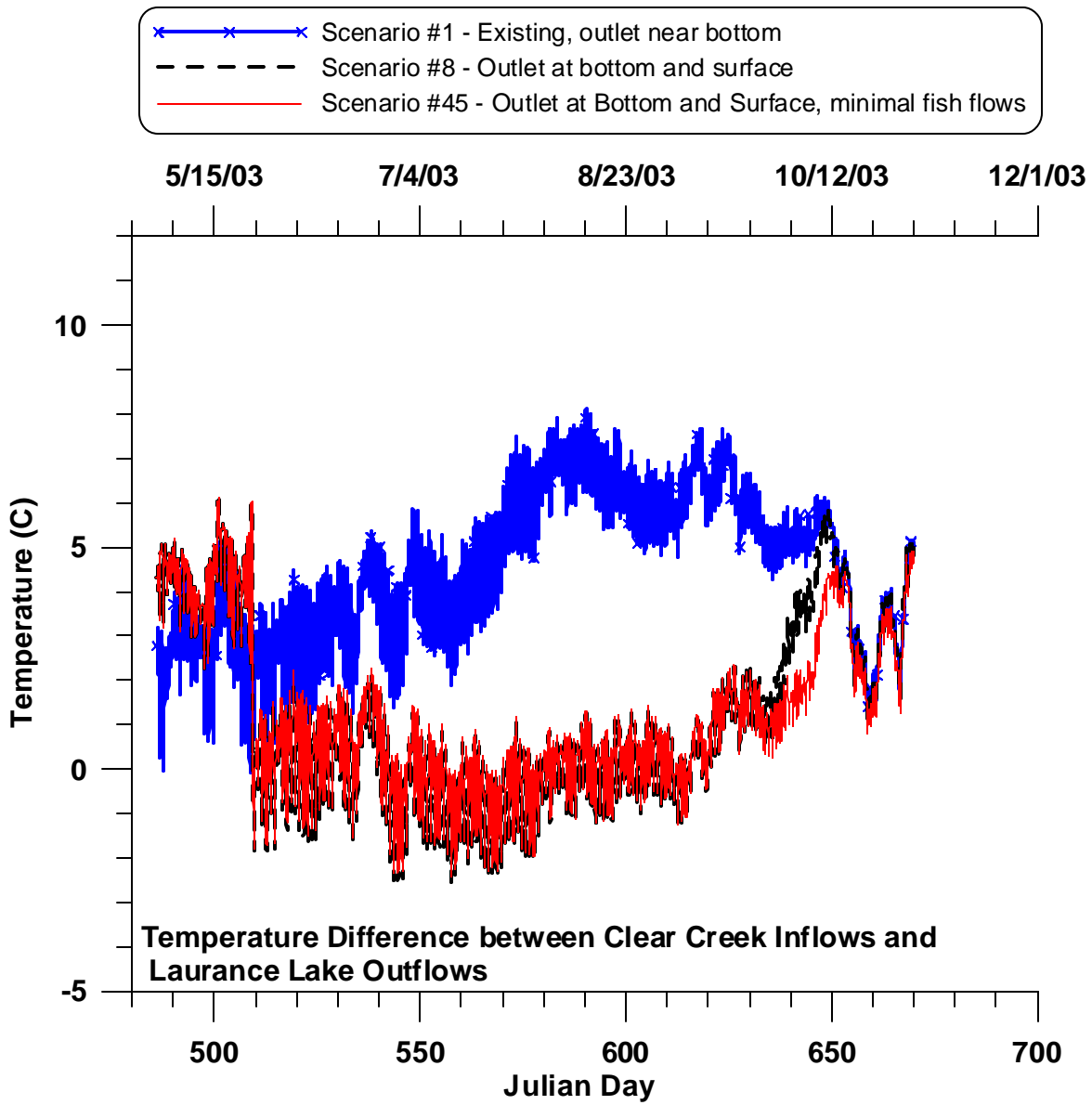


Figure 51. Predicted temperature difference between Clear Creek inflows and dam outflows to Clear Creek for scenarios 1, 8, and 45.

References

Cole, T.M., and S.A. Wells (2004) "CE-QUAL-W2: A two-dimensional, laterally averaged, Hydrodynamic and Water Quality Model, Version 3.2," Instruction Report EL-2004-, US Army Engineering and Research Development Center, Vicksburg, MS.

Edinger, J. E. and Buchak, E. M. (1978). "Numerical hydrodynamics of estuaries." *Estuarine and Wetland Processes with Special Emphasis on Modeling*, edited by P. Hamilton and K. B. MacDonald, Plenum Press, NY, 115-146

Wells, S. A. (1997) "Theoretical Basis for the CE-QUAL-W2 River Basin Model," Dept. of Civil Engr., Tech. Rpt. EWR-6-97, Portland St. Univ., Portland, OR, 1997.

Appendix A: Model Control File

W2 Model Version 3.2

TITLE CTITLE.....
 Laurance Lake Model Version 3.2
 Portland State University

Temperature simulation

Jday 1 = 1/1/2002

GRID	NWB	NBR	IMX	KMX					
	1	2	23	92					
IN/OUTFL	NTR	NST	NIW	NWD	NGT	NSP	NPI	NPU	
	1	1	0	0	0	0	0	0	
CONSTITU	NGC	NSS	NAL	NEP	NBOD				
	3	1	1	1	0				
MISCELL	NDAY								
	100								
TIME CON	TMSTRT	TMEND730	YEAR						
	486.000	851.000	2002						
DLT CON	NDT	DLTMIN							
	1	1.00000							
DLT DATE	DLTD	DLTD	DLTD	DLTD	DLTD	DLTD	DLTD	DLTD	DLTD
	63.5000								
DLT MAX	DLTMAX	DLTMAX	DLTMAX	DLTMAX	DLTMAX	DLTMAX	DLTMAX	DLTMAX	DLTMAX
	3600.00								
DLT FRN	DLTF	DLTF	DLTF	DLTF	DLTF	DLTF	DLTF	DLTF	DLTF
	0.90000								
DLT LIM1	VISC	CELC							
WB 1	ON	ON							
BRANCH G	US	DS	UHS	DHS	UQB	DQB	NLMIN	SLOPE	
BR1	2	13	0	0	0	0	1	0.00000	
BR2	16	22	0	13	0	0	1	0.00000	
LOCATION	LAT	LONG	EBOT	BS	BE	JBDN			
WB 1	45.5000	122.000	880.000	1	2	1			
INIT CND	T2I	ICEI	WTYPEC						
WB 1	-1.00000	0.00000	FRESH						
CALCULAT	VBC	EBC	MBC	PQC	EVC	PRC			
WB 1	ON	ON	ON	ON	ON	OFF			
DEAD SEA	WINDC	QINC	QOUTC	HEATC					
WB 1	ON	ON	ON	ON					
INTERPOL	QINIC	DTRIC	HDIC						
BR1	ON	ON	OFF						
BR2	ON	ON	OFF						
HEAT EXCH	SLHTC	SROC	RHEVAP	METIC	FETCHC	AFW	BFW	CFW	WINDH
WB 1	TERM	ON	OFF	ON	OFF	9.20000	0.46000	2.00000	5.0000
ICE COVE	ICEC	SLICEC	ALBEDO	HWICE	BICE	GICE	ICEMIN	ICET2	
WB 1	ON	DETAIL	0.25000	10.0000	0.60000	0.07000	0.05000	3.00000	

TRANSPOR	SLTRC	THETA								
WB 1	ULTIMATE	0.55000								
HYD COEF	AX	DX	CBHE	TSED	FI	TSEDF	FRICC			
WB 1	1.00000	1.00000	0.3	8.000	0.00000	1.00000	MANN			
EDDY VISC	AZC	AZSLC	AZMAX	PHISET						
WB 1	W2	IMP	0.00100	0.0						
N STRUC	NSTR									
BR1	1									
BR2	0									
STR INT	STRIC	STRIC	STRIC	STRIC	STRIC	STRIC	STRIC	STRIC	STRIC	STRIC
BR 1	ON									
BR 2										
STR TOP	KTSTR	KTSTR	KTSTR	KTSTR	KTSTR	KTSTR	KTSTR	KTSTR	KTSTR	KTSTR
BR1	2									
BR2										
STR BOT	KBSTR	KBSTR	KBSTR	KBSTR	KBSTR	KBSTR	KBSTR	KBSTR	KBSTR	KBSTR
BR1	91									
BR2										
STR SINK	SINKC	SINKC	SINKC	SINKC	SINKC	SINKC	SINKC	SINKC	SINKC	SINKC
BR1	POINT									
BR2										
STR ELEV	ESTR	ESTR	ESTR	ESTR	ESTR	ESTR	ESTR	ESTR	ESTR	ESTR
BR1	880.000									
BR2										
STR WIDT	WSTR	WSTR	WSTR	WSTR	WSTR	WSTR	WSTR	WSTR	WSTR	WSTR
BR1	0.00000									
BR2										
PIPES	IUPI	IDPI	EUPI	EDPI	WPI	DLXPI	FPI	FMINPI	WTHLC	
PIPE UP	PUPIC	ETUPI	EBUPI	KTUPI	KBUPI					
PIPE DOWN	PDPIC	ETDPI	EBDPI	KTDPI	KBDPI					
SPILLWAY	IUSP	IDSP	ESP	A1SP	B1SP	A2SP	B2SP	WTHLC		
SPILL UP	PUSPC	ETUSP	EBUSP	KTUSP	KBUSP					
SPILL DOWN	PDSPC	ETUSP	EBUSP	KTDSP	KBDSP					
SPILL GAS	GASSPC	EQSP	AGASSP	BGASSP	CGASSP					
GATES	IUGT	IDGT	EGT	A1GT	B1GT	G1GT	A2GT	B2GT	G2GT	WTHLC
GATE WEIR	GTA1	GTB1	GTA2	GTB2	DYNVAR					
GATE UP	PUGTC	ETUGT	EBUGT	KTUGT	KBUGT					
GATE DOWN	PDGTC	ETDGT	EBDGT	KTDGT	KBDGT					
GATE GAS	GASGTC	EQGT	AGASGT	BGASGT	CGASGT					
PUMPS 1	IUPU	IDPU	EPU	STRTPU	ENDPU	EONPU	EOFFPU	QPU	WTHLC	

PUMPS 2	PPUC	ETPU	EBPU	KTPU	KBPU				
WEIR SEG	IWR	IWR	IWR	IWR	IWR	IWR	IWR	IWR	IWR
WEIR TOP	KTWR	KTWR	KTWR	KTWR	KTWR	KTWR	KTWR	KTWR	KTWR
WEIR BOT	KBWR	KBWR	KBWR	KBWR	KBWR	KBWR	KBWR	KBWR	KBWR
WD INT	WDIC	WDIC	WDIC	WDIC	WDIC	WDIC	WDIC	WDIC	WDIC
WD SEG	IWD	IWD	IWD	IWD	IWD	IWD	IWD	IWD	IWD
WD ELEV	EWD	EWD	EWD	EWD	EWD	EWD	EWD	EWD	EWD
WD TOP	KTWD	KTWD	KTWD	KTWD	KTWD	KTWD	KTWD	KTWD	KTWD
WD BOT	KBWD	KBWD	KBWD	KBWD	KBWD	KBWD	KBWD	KBWD	KBWD
TRIB PLA	PTRC SPECIFY	PTRC	PTRC	PTRC	PTRC	PTRC	PTRC	PTRC	PTRC
TRIB INT	TRIC ON	TRIC	TRIC	TRIC	TRIC	TRIC	TRIC	TRIC	TRIC
TRIB SEG	ITR 13	ITR	ITR	ITR	ITR	ITR	ITR	ITR	ITR
TRIB TOP	ELTRT 898.0	ELTRT	ELTRT	ELTRT	ELTRT	ELTRT	ELTRT	ELTRT	ELTRT
TRIB BOT	ELTRB 880.1	ELTRB	ELTRB	ELTRB	ELTRB	ELTRB	ELTRB	ELTRB	ELTRB
DST TRIB BR 1 BR 2	DTRC OFF OFF	DTRC	DTRC	DTRC	DTRC	DTRC	DTRC	DTRC	DTRC
PUMPBACK	JBG 0	KTG 0	KBG 0	JBP 0	KTP 0	KBP 0			
PRINTER	LJC IV								
HYD PRIN NVIOL U W T RHO AZ SHEAR ST SB ADMX DM HDG ADMZ HPG GRAV	HPRWBC ON ON ON ON OFF OFF OFF OFF OFF OFF OFF OFF OFF OFF	HPRWBC	HPRWBC	HPRWBC	HPRWBC	HPRWBC	HPRWBC	HPRWBC	HPRWBC
SNP PRINT WB 1	SNPC ON	NSNP 1	NISNP 19						

SNP DATE	SNPD	SNPD	SNPD	SNPD	SNPD	SNPD	SNPD	SNPD	SNPD
WB 1	63.5000								
SNP FREQ	SNPF	SNPF	SNPF	SNPF	SNPF	SNPF	SNPF	SNPF	SNPF
WB 1	1.00000								
SNP SEG	ISNP	ISNP	ISNP	ISNP	ISNP	ISNP	ISNP	ISNP	ISNP
WB 1	2	3	4	5	6	7	8	9	10
	11	12	13	16	17	18	19	20	21
	22								
SCR PRINT	SCRC	NSCR							
WB 1	ON	1							
SCR DATE	SCRD	SCRD	SCRD	SCRD	SCRD	SCRD	SCRD	SCRD	SCRD
WB 1	63.5000								
SCR FREQ	SCRF	SCRF	SCRF	SCRF	SCRF	SCRF	SCRF	SCRF	SCRF
WB 1	0.50000								
PRF PLOT	PRFC	NPRF	NIPRF						
WB 1	OFF	1	3						
PRF DATE	PRFD	PRFD	PRFD	PRFD	PRFD	PRFD	PRFD	PRFD	PRFD
WB 1	77.7000								
PRF FREQ	PRFF	PRFF	PRFF	PRFF	PRFF	PRFF	PRFF	PRFF	PRFF
WB 1	1.00000								
PRF SEG	IPRF	IPRF	IPRF	IPRF	IPRF	IPRF	IPRF	IPRF	IPRF
WB 1	10	18	26						
SPR PLOT	SPRC	NSPR	NISPR						
WB 1	OFF	0	0						
SPR DATE	SPRD	SPRD	SPRD	SPRD	SPRD	SPRD	SPRD	SPRD	SPRD
WB 1									
SPR FREQ	SPRF	SPRF	SPRF	SPRF	SPRF	SPRF	SPRF	SPRF	SPRF
WB 1									
SPR SEG	ISPR	ISPR	ISPR	ISPR	ISPR	ISPR	ISPR	ISPR	ISPR
WB 1									
VPL PLOT	VPLC	NVPL							
WB 1	OFF	1							
VPL DATE	VPLD	VPLD	VPLD	VPLD	VPLD	VPLD	VPLD	VPLD	VPLD
WB 1	63.5000								
VPL FREQ	VPLF	VPLF	VPLF	VPLF	VPLF	VPLF	VPLF	VPLF	VPLF
WB 1	1.00000								
CPL PLOT	CPLC	NCPL							
WB 1	ON	1							
CPL DATE	CPLD	CPLD	CPLD	CPLD	CPLD	CPLD	CPLD	CPLD	CPLD
WB 1	486.400								
CPL FREQ	CPLF	CPLF	CPLF	CPLF	CPLF	CPLF	CPLF	CPLF	CPLF
WB 1	1.0								
FLUXES	FLXC	NFLX							
WB 1	OFF	0							
FLX DATE	FLXD	FLXD	FLXD	FLXD	FLXD	FLXD	FLXD	FLXD	FLXD
WB 1									
FLX FREQ	FLXF	FLXF	FLXF	FLXF	FLXF	FLXF	FLXF	FLXF	FLXF
WB 1									
TSR PLOT	TSRC	NTSR	NITSR						
	ON	1	1						

TSR DATE	TSRD 63.5000	TSRD	TSRD	TSRD	TSRD	TSRD	TSRD	TSRD	TSRD	TSRD
TSR FREQ	TSRF 0.10000	TSRF	TSRF	TSRF	TSRF	TSRF	TSRF	TSRF	TSRF	TSRF
TSR SEG	ITSR 13	ITSR	ITSR	ITSR	ITSR	ITSR	ITSR	ITSR	ITSR	ITSR
TSR LAYE	ETSR 0.00000	ETSR	ETSR	ETSR	ETSR	ETSR	ETSR	ETSR	ETSR	ETSR
WITH OUT	WDOC ON	NWDO 1	NIWDO 1							
WITH DAT	WDOD 1.00000	WDOD	WDOD	WDOD	WDOD	WDOD	WDOD	WDOD	WDOD	WDOD
WITH FRE	WDOF 0.00100	WDOF	WDOF	WDOF	WDOF	WDOF	WDOF	WDOF	WDOF	WDOF
WITH SEG	IWDO 13	IWDO	IWDO	IWDO	IWDO	IWDO	IWDO	IWDO	IWDO	IWDO
RESTART	RSOC OFF	NRSO 0	RSIC OFF							
RSO DATE	RSOD	RSOD	RSOD	RSOD	RSOD	RSOD	RSOD	RSOD	RSOD	RSOD
RSO FREQ	RSOF	RSOF	RSOF	RSOF	RSOF	RSOF	RSOF	RSOF	RSOF	RSOF
CST COMP	CCC OFF	LIMC OFF	CUF 1							
CST ACTIVE	CAC									
TDS	ON									
Gen1	ON									
Gen2	ON									
Gen3	ON									
ISS1	ON									
PO4	ON									
NH4	ON									
NO3	ON									
DSI	OFF									
PSI	OFF									
FE	ON									
LDOM	ON									
RDOM	ON									
LPOM	ON									
RPOM	OFF									
ALG1	ON									
DO	ON									
TIC	ON									
ALK	ON									
CST DERI	CDWBC	CDWBC	CDWBC	CDWBC	CDWBC	CDWBC	CDWBC	CDWBC	CDWBC	CDWBC
DOC	OFF									
POC	OFF									
TOC	OFF									
DON	OFF									
PON	OFF									
TON	OFF									
TKN	OFF									
TN	OFF									
DOP	OFF									
POP	OFF									
TOP	OFF									
TP	OFF									
APR	OFF									
CHLA	OFF									
ATOT	OFF									
%DO	OFF									

TSS	OFF
TISS	OFF
CBOD	OFF
pH	OFF
CO2	OFF
HCO3	OFF
CO3	OFF

CST FLUX	CFWBC	CFWBC	CFWBC	CFWBC	CFWBC	CFWBC	CFWBC	CFWBC	CFWBC
TISSIN	OFF								
TISSOUT	OFF								
PO4AR	OFF								
PO4AG	OFF								
PO4AP	OFF								
PO4ER	OFF								
PO4EG	OFF								
PO4EP	OFF								
PO4POM	OFF								
PO4DOM	OFF								
PO4OM	OFF								
PO4SED	OFF								
PO4SOD	OFF								
PO4SET	OFF								
NH4NITR	OFF								
NH4AR	OFF								
NH4AG	OFF								
NH4AP	OFF								
NH4ER	OFF								
NH4EG	OFF								
NH4EP	OFF								
NH4POM	OFF								
NH4DOM	OFF								
NH4OM	OFF								
NH4SED	OFF								
NH4SOD	OFF								
NO3DEN	OFF								
NO3AG	OFF								
NO3EG	OFF								
NO3SED	OFF								
DSIAG	OFF								
DSIEG	OFF								
DSIPIS	OFF								
DSISED	OFF								
DSISOD	OFF								
DSISET	OFF								
PSIAM	OFF								
PSINET	OFF								
PSIDK	OFF								
FESET	OFF								
FESED	OFF								
LDOMDK	OFF								
LRDOM	OFF								
RDOMDK	OFF								
LDOMAP	OFF								
LDOMEF	OFF								
LPOMDK	OFF								
LRPOM	OFF								
RPOMDK	OFF								
LPOMAP	OFF								
LPOMEF	OFF								
LPOMSET	OFF								
RPOMSET	OFF								
CBODDK	OFF								
DOAP	OFF								
DOAR	OFF								
DOEP	OFF								
DOER	OFF								
DOPOM	OFF								
DODOM	OFF								
DOOM	OFF								
DONITR	OFF								
DOCBOD	OFF								
DOREAR	OFF								
DOSED	OFF								

DOSOD OFF
 TICAG OFF
 TICEG OFF
 SEDDK OFF
 SEDAS OFF
 SEDLPOM OFF
 SEDSET OFF
 SODDK OFF

CST	ICON	C2IWB	C2IWB	C2IWB	C2IWB	C2IWB	C2IWB	C2IWB	C2IWB	C2IWB
TDS		51.0000								
Gen1		100.000								
Gen2		0.00000								
Gen3		10.0000								
ISS1		2.00000								
PO4		0.00000								
NH4		0.00000								
NO3		0.14000								
DSI		0.00000								
PSI		0.00000								
FE		0.10000								
LDOM		0.70000								
RDOM		2.02000								
LPOM		0.10000								
RPOM		0.00000								
ALG1		1.00000								
DO		1.00000								
TIC		11.9100								
ALK		31.0000								

CST	PRIN	CPRWBC	CPRWBC	CPRWBC	CPRWBC	CPRWBC	CPRWBC	CPRWBC	CPRWBC	CPRWBC
TDS		ON								
Gen1		ON								
Gen2		OFF								
Gen3		OFF								
ISS1		ON								
PO4		ON								
NH4		ON								
NO3		ON								
DSI		OFF								
PSI		OFF								
FE		ON								
LDOM		ON								
RDOM		ON								
LPOM		ON								
RPOM		OFF								
ALG1		ON								
DO		ON								
TIC		OFF								
ALK		OFF								

CIN	CON	CINBRC	CINBRC	CINBRC	CINBRC	CINBRC	CINBRC	CINBRC	CINBRC	CINBRC
TDS		ON	ON							
Gen1		ON	ON							
Gen2		OFF	OFF							
Gen3		ON	ON							
ISS1		ON	ON							
PO4		ON	ON							
NH4		ON	ON							
NO3		ON	ON							
DSI		OFF	OFF							
PSI		OFF	OFF							
FE		ON	ON							
LDOM		ON	ON							
RDOM		ON	ON							
LPOM		ON	ON							
RPOM		OFF	OFF							
ALG1		ON	ON							
DO		ON	ON							
TIC		ON	ON							
ALK		ON	ON							

CTR	CON	CTRTRC	CTRTRC	CTRTRC	CTRTRC	CTRTRC	CTRTRC	CTRTRC	CTRTRC	CTRTRC
TDS		OFF								

Gen1	OFF
Gen2	OFF
Gen3	OFF
ISS1	OFF
PO4	OFF
NH4	OFF
NO3	OFF
DSI	OFF
PSI	OFF
FE	OFF
LDOM	OFF
RDOM	OFF
LPOM	OFF
RPOM	OFF
ALG1	OFF
DO	OFF
TIC	OFF
ALK	OFF

CDT CON	CDTBRC	CDTBRC	CDTBRC	CDTBRC	CDTBRC	CDTBRC	CDTBRC	CDTBRC	CDTBRC	CDTBRC
TDS	OFF	OFF								
Gen1	OFF	OFF								
Gen2	OFF	OFF								
Gen3	OFF	OFF								
ISS1	OFF	OFF								
PO4	OFF	OFF								
NH4	OFF	OFF								
NO3	OFF	OFF								
DSI	OFF	OFF								
PSI	OFF	OFF								
FE	OFF	OFF								
LDOM	OFF	OFF								
RDOM	OFF	OFF								
LPOM	OFF	OFF								
RPOM	OFF	OFF								
ALG1	OFF	OFF								
DO	OFF	OFF								
TIC	OFF	OFF								
ALK	OFF	OFF								

CPR CON	CPRBRC	CPRBRC	CPRBRC	CPRBRC	CPRBRC	CPRBRC	CPRBRC	CPRBRC	CPRBRC	CPRBRC
TDS	OFF	OFF								
Gen1	OFF	OFF								
Gen2	OFF	OFF								
Gen3	OFF	OFF								
ISS1	OFF	OFF								
PO4	OFF	OFF								
NH4	OFF	OFF								
NO3	OFF	OFF								
DSI	OFF	OFF								
PSI	OFF	OFF								
FE	OFF	OFF								
LDOM	OFF	OFF								
RDOM	OFF	OFF								
LPOM	OFF	OFF								
RPOM	OFF	OFF								
ALG1	OFF	OFF								
DO	OFF	OFF								
TIC	OFF	OFF								
ALK	OFF	OFF								

EX COEF	EXH2O	EXSS	EXOM	BETA	EXC	EXIC
WB 1	0.55000	0.01000	0.20000	0.45000	ON	ON

ALG EX	EXA	EXA	EXA	EXA	EXA	EXA
	0.20000					

GENERIC	CGQ10	CG0DK	CG1DK	CGS
CG 1	0.00000	0.00000	0.00000	0.00000
CG 2	0.00000	-1.00000	0.00000	0.00000
CG 3	1.04000	0.00000	1.40000	0.00000

S SOLIDS	SSS	SEDRC	TAUCR
SS# 1	1.00000	OFF	.15E-04

ALGAL RATE	AG	AR	AE	AM	AS	AHSP	AHSN	AHSSI	ASAT
ALG1	2.00000	0.04000	0.04000	0.10000	0.10000	0.00300	0.01400	0.00000	100.000
ALGAL TEMP	AT1	AT2	AT3	AT4	AK1	AK2	AK3	AK4	
ALG1	5.00000	30.0000	35.0000	40.0000	0.10000	0.99000	0.99000	0.10000	
ALG STOI	ALGP	ALGN	ALGC	ALGSI	ACHLA	ALPOM	ANEQN	ANPR	
ALG1	0.00500	0.08000	0.45000	0.00000	65.0000	0.80000	1	0.00000	
EPIPHYTE	EPIC	EPIC	EPIC	EPIC	EPIC	EPIC	EPIC	EPIC	EPIC
EPI1	OFF								
EPI PRIN	EPRC	EPRC	EPRC	EPRC	EPRC	EPRC	EPRC	EPRC	EPRC
EPI1	OFF								
EPI INIT	EPICI	EPICI	EPICI	EPICI	EPICI	EPICI	EPICI	EPICI	EPICI
EPI1	0.00000								
EPI RATE	EG	ER	EE	EM	EB	EHSP	EHSN	EHSSI	
EPI1	2.00000	0.04000	0.04000	0.10000	0.00100	0.00300	0.01400	0.00000	
EPI HALF	ESAT	EHS	ENEQN	ENPR					
EPI1	125.000	1.00000	1	0.00000					
EPI TEMP	ET1	ET2	ET3	ET4	EK1	EK2	EK3	EK4	
EPI1	5.00000	25.0000	35.0000	40.0000	0.10000	0.99000	0.99000	0.10000	
EPI STOI	EP	EN	EC	ESI	ECHLA	EPOM			
EPI1	0.00500	0.08000	0.45000	0.00000	65.0000	0.80000			
DOM	LDOMDK	RDOMDK	LRDDK						
WB 1	0.30000	0.00100	0.01000						
POM	LPOMDK	RPOMDK	LRPDK	POMS					
WB 1	0.08000	0.01000	0.00100	0.50000					
OM STOIC	ORGP	ORGN	ORGC	ORGSI					
WB 1	0.00500	0.08000	0.45000	0.18000					
OM RATE	OMT1	OMT2	OMK1	OMK2					
WB 1	4.00000	30.0000	0.10000	0.99000					
CBOD	KBOD	TBOD	RBOD						
BOD 1	0.25000	1.01500	1.85000						
CBOD STOIC	BODP	BODN	BODC						
BOD 1	0.00500	0.08000	0.45000						
PHOSPHOR	PO4R	PARTP							
WB 1	0.01500	1.20000							
AMMONIUM	NH4R	NH4DK							
WB 1	0.15000	0.05000							
NH4 RATE	NH4T1	NH4T2	NH4K1	NH4K2					
WB 1	5.00000	25.0000	0.10000	0.99000					
NITRATE	NO3DK	NO3S							
WB 1	0.05000	0.00000							
NO3 RATE	NO3T1	NO3T2	NO3K1	NO3K2					
WB 1	5.00000	25.0000	0.10000	0.99000					
SILICA	DSIR	PSIS	PSIDK	PARTSI					
WB 1	0.10000	0.10000	0.30000	0.20000					
IRON	FER	FES							
WB 1	0.50000	2.00000							
SED CO2	CO2R								
WB 1	0.10000								
STOICH 1	O2NH4	O2OM							

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WB 1      4.57000 1.40000

STOICH 2   O2AR   O2AG
ALG1      1.10000 1.40000

STOICH 3   O2ER   O2EG
EPI1      1.10000 1.40000

O2 LIMIT   O2LIM
           0.10000

SEDIMENT   SEDC   SEDPRC  SEDCI   SEDK   FSOD   FSED
WB 1       ON     ON     1.00000 0.08000 1.00000 1.00000

SOD RATE   SODT1  SODT2  SODK1  SODK2
WB 1       4.00000 30.0000 0.10000 0.99000

S DEMAND   SOD     SOD     SOD     SOD     SOD     SOD     SOD     SOD     SOD
           0.30000 0.30000 0.30000 0.30000 0.30000 0.40000 0.50000 0.50000 0.50000
           0.70000 0.90000 1.10000 1.30000 1.50000 1.70000 1.90000 1.90000 1.90000
           1.70000 1.50000 1.40000 1.30000 0.00000

REAERATION TYPE   EQN#   COEF1   COEF2   COEF3   COEF4
WB 1       LAKE   2   0.00000 0.00000 0.00000 0.00000

RSI FILE.....RSIFN.....
    rsi.npt

QWD FILE.....QWDFN.....
    qwd.npt

QGT FILE.....QGTFN.....
    qgt.npt

WSC FILE.....WSCFN.....
    wsc.npt

SHD FILE.....SHDFN.....
    shd.npt

BTH FILE.....BTHFN.....
WB 1    bth.npt

MET FILE.....METFN.....
WB 1    met.npt

EXT FILE.....EXTFN.....
WB 1    ext.npt

VPR FILE.....VPRFN.....
WB 1    vpr.npt

LPR FILE.....LPRFN.....
WB 1    lpr.npt - not used

QIN FILE.....QINFN.....
BR1    qin_br1.npt
BR2    qin_br2.npt

TIN FILE.....TINFN.....
BR1    tin_br1.npt
BR2    tin_br2.npt

CIN FILE.....CINFN.....
BR1    cin_br1.npt
BR2    cin_br2.npt

QOT FILE.....QOTFN.....
BR1    qout.npt
BR2    qot_br2.npt

QTR FILE.....QTRFN.....
TR1    qwb.npt

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TTR FILE.....TTRFN.....
TR1    tdt_br1.npt

CTR FILE.....CTRFN.....
TR1    ctr_tr1.npt - not used

QDT FILE.....QDTFN.....
BR1    qwb.npt
BR2    qdt_br2.npt - not used

TDT FILE.....TDTFN.....
BR1    tdt_br1.npt
BR2    tdt_br2.npt - not used

CDT FILE.....CDTFN.....
BR1    cdt_br1.npt - not used
BR2    cdt_br2.npt - not used

PRE FILE.....PREFN.....
BR1    pre_br1.npt - not used
BR2    pre_br2.npt

TPR FILE.....TPRFN.....
BR1    tpr_br1.npt - not used
BR2    tpr_br2.npt

CPR FILE.....CPRFN.....
BR1    cpr_br1.npt - not used
BR2    cpr_br2.npt

EUH FILE.....EUHFN.....
BR1    euh_br1.npt - not used
BR2    euh_br2.npt

TUH FILE.....TUHFN.....
BR1    tuh_br1.npt - not used
BR2    tuh_br2.npt

CUH FILE.....CUHFN.....
BR1    cuh_br1.npt - not used
BR2    cuh_br2.npt

EDH FILE.....EDHFN.....
BR1    edh_br1.npt - not used
BR2    edh_br2.npt

TDH FILE.....TDHFN.....
BR1    tdh_br1.npt - not used
BR2    tdh_br2.npt

CDH FILE.....CDHFN.....
BR1    cdh_br1.npt - not used
BR2    cdh_br2.npt

SNP FILE.....SNPFN.....
WB 1   snp_wb1.opt

PRF FILE.....PRFFN.....
WB 1   prf_wb1.opt - not used

VPL FILE.....VPLFN.....
WB 1   vpl_wb1.opt - not used

CPL FILE.....CPLFN.....
WB 1   cpl_wb1.opt

SPR FILE.....SPRFN.....
WB 1   spr_wb1.opt - not used

FLX FILE.....FLXFN.....
WB 1   flx_wb1.opt - not used

TSR FILE.....TSRFN.....
      tsr_wb1.opt

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WDO FILE.....WDOFN.....
wdo_wb1.opt